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# LiDAR Project Report

G16PD00019, Middle Brazos  
Lake Whitney, TX QL2 LiDAR

## QL2 LiDAR

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Prepared For:

United States Geological Survey



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TASK ORDER: # G16PD00019

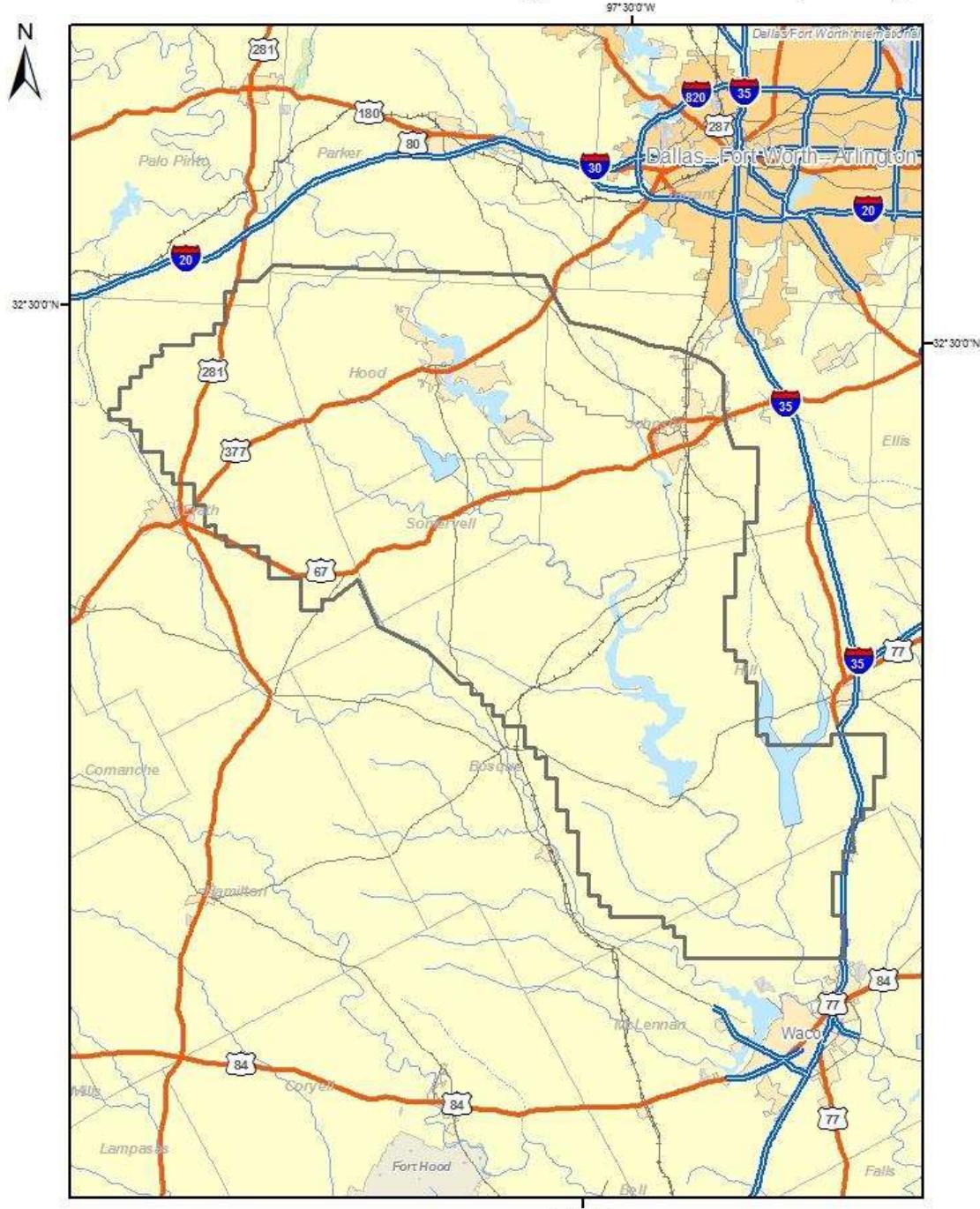
Project Report  
LiDAR Collection, Processing, and QA/QC

G16PD00019, Middle Brazos Lake  
Whitney, TX QL2 LiDAR

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## Middle Brazos - Lake Whitney, TX QL2 Lidar (FY15)



FEMA Region 6 QL2 Lidar AOI

 FY15\_MiddleBrazos\_LakeWhitney\_LIDAR\_AOI\_Final

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## 1 Introduction and Specifications

Digital Aerial Solutions, LLC (DAS) was tasked to collect and process a Light Detection And Ranging (LiDAR) derived elevation dataset for the G16PD00019, Middle Brazos Lake Whitney, TX QL2 LiDAR. The area encompasses approximately 2370 square miles. Aerial LiDAR data was collected utilizing an ALS70 and ALS80. The ALS70 is a discrete return topographic LiDAR mapping system manufactured by Leica Geosystems.

LiDAR data collected for the G16PD00019, Middle Brazos Lake Whitney, TX Lidar survey has a nominal pulse spacing of 0.7 meters, and includes up to 8 discrete returns per pulse, along with intensity values for each return.

LiDAR datasets were post processed to generate elevation point cloud swaths for each flight line. Deliverables include the point cloud swaths, tiled point clouds classified by land cover type, breaklines to support hydro-flattening of digital elevation models (DEM)s, intensity tiles, and bare-earth DEM tiles. Point cloud deliverables are stored in the LAS version 1.4 format, point data record format 6. The tiling scheme for tiled deliverables is a 1500 meters x 1500 meter grid. Tile number is the appropriate cell number values found in the USNG index. All deliverables were generated in conformance with the *U.S. Geological Survey National Geospatial Program Guidelines and Base Specifications, Version 1.2*.

## 2 Spatial Reference System

The spatial reference of the data is as follows.

### Horizontal Spatial Reference

- Datum: NAVD88, Meters (to 3 decimal places)
- Coordinates: UTM Zone 14, NAD83, Meters (to 2 decimal places);

### Vertical Spatial Reference

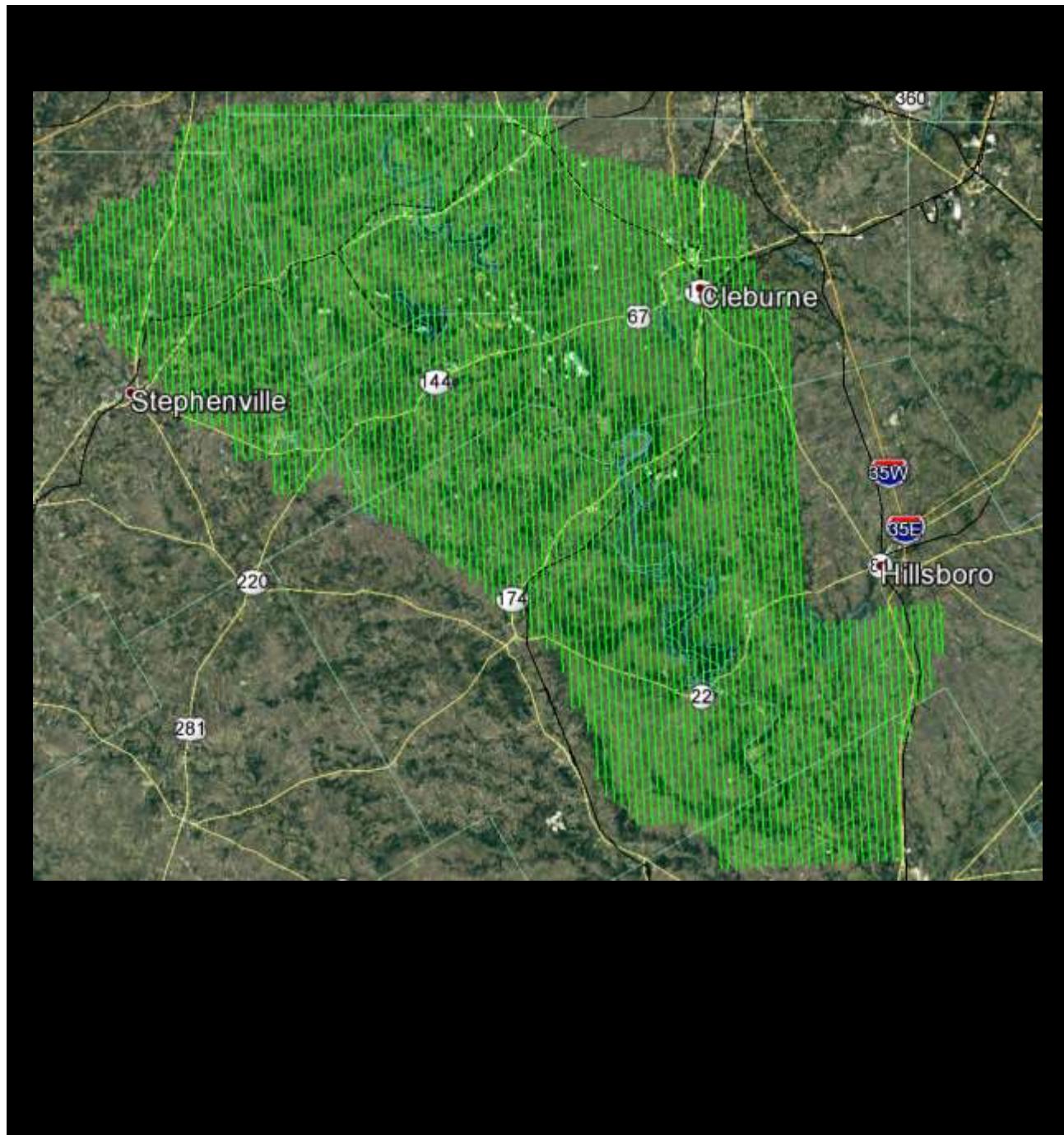
*All datasets are available with orthometric elevation; point cloud datasets are also available with ellipsoid heights*

- Datum: North American Vertical Datum of 1983 (GEOID12B)

### 3 LiDAR Acquisition

#### 3.1 Survey Area

The G16PD00019, Middle Brazos Lake Whitney, TX QL2 LiDAR survey covers approximately 3993 square miles covering Erath, Hood, Johnson, Somerwell, Bosque, Hill and McLennan Counties. The flight plan consisted of 118 survey lines and 3 control lines.



### 3.2 Acquisition Parameters

Acquisition parameters include the sensor configuration and the flight plan characteristics, and are selected based on a number of project specific criteria. Criteria reviewed include the required accuracies for the final dataset, the land cover types within the project survey area, and the required nominal pulse spacing. Acquisition parameters selected for the Middle Brazos Lake Whitney 2ppsm Lidar project are summarized below.

Parameter	Value
Flying Height Above Ground Level	6230 feet
Nominal Sidelap	30%
Nominal Speed Over Ground	155 knots
Field of View	36°
Laser Rate	132 kHz
Scan Rate	66.2 hz
Maximum Cross Track Spacing	0.78 meters
Maximum Along Track Spacing	0.82 meters
Average Spacing	0.7 meters

### 3.3 Acquisition Mission

The acquisition mission for G16PD00019, Middle Brazos Lake Whitney, TX QL2 LiDAR survey was coordinated to be acquired in 2 weeks, Collection began on March 1st 2016 and was completed on March 21th, 2016.

### 3.4 Airborne GPS/IMU

Airborne global positioning system (GPS) and inertial measurement unit (IMU) data was collected on the aircraft during the acquisition mission, providing sensor position and orientation information for geo-referencing the LiDAR data. Airborne GPS observations were collected at a frequency of 2Hz, and IMU observations are collected at a frequency of 200Hz.

Aircraft	Sensor	GPS Lever Arm (m)	IMU Lever Arm (m)
C421 - N112MJ	ALS70 - SN1132	x: -0.210, y: -0.060, z: -1.370	x: -0.450, y: -0.159, z: -0.169

In addition, GPS data was collected with ground base stations during the acquisition mission, providing corrections to support differential post-processing of the airborne GPS. One ground base station was setup at an NGS Benchmark as the base of operation. The additional ground base station were selected and place threw the project to ensure complete coverage. Ground GPS observations were collected at a frequency of 2Hz.

## 4 LiDAR Processing

### 4.1 Acquisition Post-Processing

Once the acquisition was completed, initial post-processing was performed to generate geo-referenced LiDAR elevation point clouds.

The airborne GPS dataset was differentially corrected using the ground base station GPS datasets collected by DAS in Leica's INERTIAEXPLORE software. INERTIAEXPLORE computes the GPS dataset corrections in both forward and reverse chronological sequence, obtaining two solutions for the GPS trajectory. The differences between these two solutions were reviewed to ensure a consistent result, and agree within +/- 3cm. The forward and reverse solutions also show good fit between the two different base stations used in the post-processing.

Differentially corrected airborne GPS data was merged with the airborne IMU dataset in Leica's INERTIAEXPLORE software through Kalman filtering techniques. INERTIAEXPLORE applies the reference lever arms for the GPS and IMU measurement systems during processing to determine the trajectory (position and orientation) of the LiDAR sensor during the acquisition mission. Estimated lever arm values reported posteriori validate the measurements made during sensor installation in the aircraft.

Raw LiDAR sensor ranging data and the final sensor trajectory from INERTIAEXPLORE were processed in Leica's ALSPP software to produce the LiDAR elevation point cloud swaths for each flightline, stored in LAS version 1.4 file format. Quality control of the swath point clouds was performed to validate proper function of the sensor systems, full coverage of the project AOI, and point density consistent with the planned nominal pulse spacing.

Swath point clouds were assigned a unique File Source ID within the LAS file format before further processing. Swath files for the G16PD00019, Middle Brazos Lake Whitney, TX QL2 LiDAR project were numbered in chronological order of acquisition.

### 4.2 Geometric Calibration

Geometric and positional accuracy of the LiDAR swath point clouds is highly dependent on accurate calibration of the various subsystems within the LiDAR sensor system. Sensor calibration parameters fall into two categories, one being those parameters proprietary to the manufacturer's sensor design, and the other being parameters common to most commercial airborne LiDAR sensors, the IMU to laser reference system alignment angles (bore-site), and mirror deformation constants (scaling).

The manufacturer specific calibration parameters are applied in Leica's Cloud Pro software for the ALS80 sensor system. Terrasolid's Terramatch software was used to calculate the IMU bore-site and mirror scale parameters for the G16PD00019, Middle Brazos Lake Whitney, TX QL2 LiDAR. Within the TerraMatch software, the Tie-line workflow was used to solve for the parameters. The Tie-line workflow involves automated selection of numerous 'tie-lines', which represent a linear segment fit to the data that should have the same slope, azimuth, position and elevation, within the overlap sections of the survey lines and control lines. The tie-lines provide observations for algorithms within TerraMatch to solve for the bore-site and mirror scale parameters for the lift.

The Tie-line workflow is dependent upon well distributed tie-lines throughout the swath point clouds to effectively solve for bore-site and mirror scale parameters with the automated algorithms.

survey and control lines. Manual estimation of the bore-site and mirror scale parameters was performed using the observed tie-lines in overlap areas.

The final step of geometric calibration is to determine elevation (z) offset corrections to be applied to the swath point clouds. Z values calculated during the course of the acquisition mission can vary at the centimeter level as the GPS satellite constellation observed in the survey area changes with satellites moving through their orbits over the course of the mission. Baseline length from the ground base station GPS to the airborne GPS can also impact the z values calculated for the swath point clouds. Z offset corrections are calculated in two steps; a relative step, where individual lines are corrected one to another using the adjusted tie-lines from the bore-site and mirror scale calculation step; and an absolute step, where groups of lines are leveled to project ground control.

For the G16PD00019, Middle Brazos Lake Whitney, TX QL2 LiDAR project, the control lines were used to determine relative z offset corrections in areas of discernible ground. The base station operated by DAS in the survey area provided for minimal baseline lengths, resulting in generally good z agreement between the survey lines and control lines.

The final geometrically calibrated swath point clouds were compared to the bare-earth profile survey data. The data fit the profile surveys within the vertical accuracy tolerance specified for the project. Full documentation of the vertical accuracy checks maybe found in section 5.1.

#### 4.3 Point Cloud Classification

Georeference information was applied to the swath point could LAS files. Geometrically calibrated swath point clouds were cut into USNG index, 1500 meter x 1500 meter LAS 1.4 format tiles for point cloud classification and derived product creation.

Tiled point cloud data was processed in Terrasolid's Terrascan software to assign initial classification values. The Terrascan software provides a number of routines to algorithmically detect and assign points to their appropriate class. Points left unclassified by the algorithmic routine remain as Class 1 – Processed, but unclassified. Automated classification routines assigned points to one of the following classes:

- Class 1 – Processed, but unclassified
- Class 2 – Bare-earth ground
- Class 7 – Low Noise (low, manually identified, if necessary)
- Class 9 — Water
- Class 10 — Ignored Ground (Breakline Proximity)
- Class 17 — Bride Decks
- Class 18 – High Noise (high, manually identified, if necessary)

Automated classification results were reviewed for each tiled point cloud, and manual edits made where necessary to correct for misclassified points. Points remaining in Class 1 after the automated classification routines were run were left in Class 1. Points falling outside of a 100 meter buffer of the project AOI polygon were excluded from the tiled point clouds.

## 4.4 Breakline Collection

Manual breakline collection was performed to support the hydro-flattening requirements of the project's DEM deliverables. Breaklines were collected directly from the classified point clouds and from triangulated irregular network (TIN) surface models built from the classified point clouds, in Terrasolids's Terrascan and Terramodeler software. Breakline features were collected as design file elements in Bentley's Microstation software. Breaklines were converted to ESRI 3D shapefile format for the breakline deliverable, and tiled to USNG index.

The data collected for the G16PD00019, Middle Brazos Lake Whitney, TX QL2 LiDAR survey maintained significant point density in the water, limiting the usefulness of point density as guiding factor in breakline placement.

Points classified as Class 2 – Bare-earth ground, falling within a one meter buffer of the collected breaklines, were reassigned to Class 10 – Ignored Ground. These points are excluded from the surface model during DEM generation to preserve the hydro-flattening characteristics of the breaklines.

## 4.5 DEM Generation

The final classified point clouds and collected breaklines were reviewed for completeness and conformance to the task order scope of work. Within the Terramodeler software, points in Class 2 – Bare-earth ground and the breaklines were combined to generate TIN elevation models for each tile, from which the bare-earth DEM tiles were interpolated and exported as 32 bit raster IMG format.

# 5 Quality Control

## 5.1 Point Clouds

Accuracy and completeness of the LiDAR point clouds directly impacts the quality of all other derived LiDAR derived products. Ensuring a quality LiDAR dataset begins with proper mission planning and execution. Ground GPS base stations are located such that GPS baselines between the ground and airborne receivers do not exceed 30km. For the G16PD00019, Middle Brazos Lake Whitney, TX QL2 LiDAR project, two base stations were run to meet this requirement, one at the field operations airport and one within the survey area. Static alignment is performed both before take-off and after landing to allow for GPS integer ambiguity resolution. Sensor operators carefully monitor the LiDAR unit and its various subsystems during the acquisition mission to ensure proper function. Airborne GPS positional dilution of precision (PDOP) estimates are monitored to ensure they remain less than 3. The optical system is monitored to ensure there are no ranging errors encountered during the flight lines.

During acquisition post-processing estimates of the trajectory data accuracy are reviewed to ensure they will support the required accuracies of the point cloud data. The trajectory accuracy is a function of the differentially corrected GPS data and the IMU data.

The raw swath point clouds generated from CloudPro are reviewed as another check for proper sensor function. The point clouds are reviewed for full coverage of the AOI, required point density and nominal pulse spacing, clustering, proper intensity values, full swath coverage within the planned field of view, and planned survey line overlap.

Geometric calibration quality control validates that the positional accuracy requirements of the project are met, and includes relative accuracy assessments for intra-swath (within) and inter-swath (between) accuracy, along with absolute accuracy assessments against project ground control.

Relative vertical accuracy assessments are normally made using the tie-lines generated in the Terramatch software, as these lines provide positional observations throughout the extent of individual swaths, and between neighboring swaths.

There is not a systematic method of testing when testing horizontal accuracy in LiDAR. The estimated Horizontal accuracy at one sigma based on the flying height for the project, is between 10cm and 20cm according to manufacturer specifications.

Absolute vertical accuracy assessments for the point cloud data are made against ground check point data. For the G16PD00019, Middle Brazos Lake Whitney, TX QL2 LiDAR, ground check point data consisted of the ground GPS base station, and real-time kinematic (RTK) GPS techniques.

Check point locations were collected at 1 – second intervals during the RTK survey. Points collected during the static pre-initialization and post-initialization were removed from the assessment so as not to bias the assessment.

Local TIN models of the elevation points are built around each ground check points. The tin model elevation is sampled at the horizontal position of the ground check point. The TIN model elevation and ground check point survey elevation values were used to calculate the fundamental vertical accuracy (FVA) of the swath point clouds. The NVA of the TIN tested RMSE<sub>Z</sub> 0.036 meters and 0.070 meters at the 95% confidence level in open terrain. NVA of the DEM tested at an RMSE<sub>Z</sub> of 0.035 meters and 0.069 meters at the 95% confidence level in open terrain. The full calculations for all check points can be found in Appendix B.

FVA of TIN

RMSE <sub>Z</sub> =	0.036	meters
NSSDA=	0.070	meters

FVA of DEM

RMSE <sub>Z</sub> =	0.035	meters
NSSDA=	0.069	meters

The tiled point cloud products were reviewed for full coverage of the AOI and proper classification. As part of the QC process, TINs are built in the Terramodeler software for each tile using the ground class and the hydro-flattening breaklines. The TINs are reviewed for non-ground features, and edited where necessary to remove any remaining non-ground features. Points were also reviewed for absolute elevation, and points falling below the selected orthometric elevation for water were removed from the ground class.

## 5.2 Breaklines

The final breaklines in ESRI 3D shapefile format were reviewed for topological consistency and correct elevation. Breaklines features are continuous and do not have overlaps or dangles.

## 5.3 Digital Elevation Models

Digital elevation models (DEMs) were reviewed for conformance with the SOW and the Base Mapping Specification version 1.2 guidelines. DEM files were loaded in the Global Mapper software and inspected visually for edge matching between tiles, void areas within the project AOI, and proper coding of the NODATA values. DEM file naming was verified for consistency with the USNG index.

## Appendix A. Flight Logs



























ALS80 LiDAR Flight Log





ALS80 LiDAR Flight Log				ALS80		SN 8235														
Project		TN3CountyQL2										Sensor Operator/s Bertin Evina-Ze								
Date/Julian:		1/5/2016				Disk Drive MM70						Base PID:		Pilot/s						
Hobbs End		2964.1				1-808654A		155				FE2752		MWAZ						
Hobbs ST		2959.6		LIFT				TAR ALT AGL (ft):		Flight Plan(s):		Base Height:		Aircraft	Airport Idnt:					
Flight Time		4.5		A				6,300		TN_ALS80		1.500		421C 13RF	M08 (Bolivar, TN)					
Lift	Flight Line	Mission	Line	UTC time:		GPS Altitude: ASL:	Direction	Speed: kts:	Available MM Space	S/Vs:	Position Acc.		Comments and Conditions:							
				B:	E:						PDOP	HDOP								
<b>A</b>									392											
1	28	160105_180908		18:09	18:19	6,300	180	156	348	18	1.3	0.7								
2	27	160105_182406		18:24	18:34		0	157	344	19	1.2	0.6								
3	26	160105_183846		18:38	18:48		180	156	338	20	1.1	0.6								
4	25	160105_185347		18:53	19:03		0	158	334	20	1.1	0.6								
5	24	160105_190826		19:08	19:18		180	156	330	20	1.0	0.6								
6	23	160105_192249		19:22	19:32		0	156	326	19	1.0	0.6								
7	22	160105_193736		19:37	19:48		180	155	322	21	1.0	0.5								
8	21	160105_195210		19:52	20:02		0	157	318	20	1.0	0.6								
9	20	160105_201025		20:11	20:20		180	154	314	19	1.1	0.6								
10	19	160105_202440		20:24	20:34		0	157	310	18	1.2	0.6								
11	X01	160105_204121		20:41	20:46		270	155	306	18	1.2	0.7	X-STRIP							
12	18	160105_205115		20:51	21:01		180	156	302	17	1.2	0.7								
13	X04	160105_210758		21:08	21:11		90	156	300	17	1.1	0.7	X-STRIP							
14	17	160105_211537		21:15	21:25		0	155	296	19	1.0	0.6								
15	16	160105_213021		21:30	21:40		180	157	292	18	1.0	0.6								
16	15	160105_214529		21:45	21:55		0	155	287	16	1.3	0.7								



















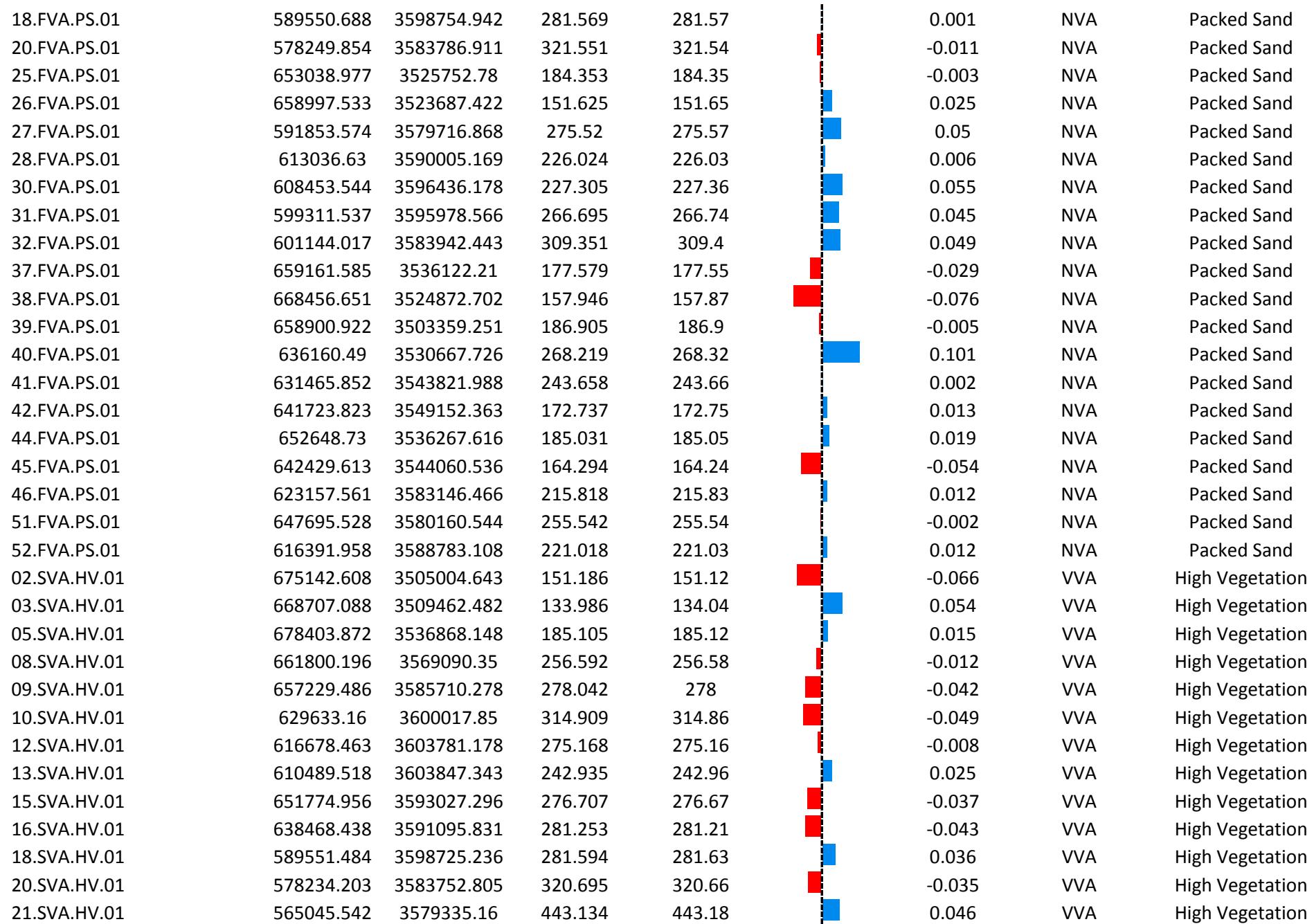
## Appendix B. Vertical Accuracy Calculations

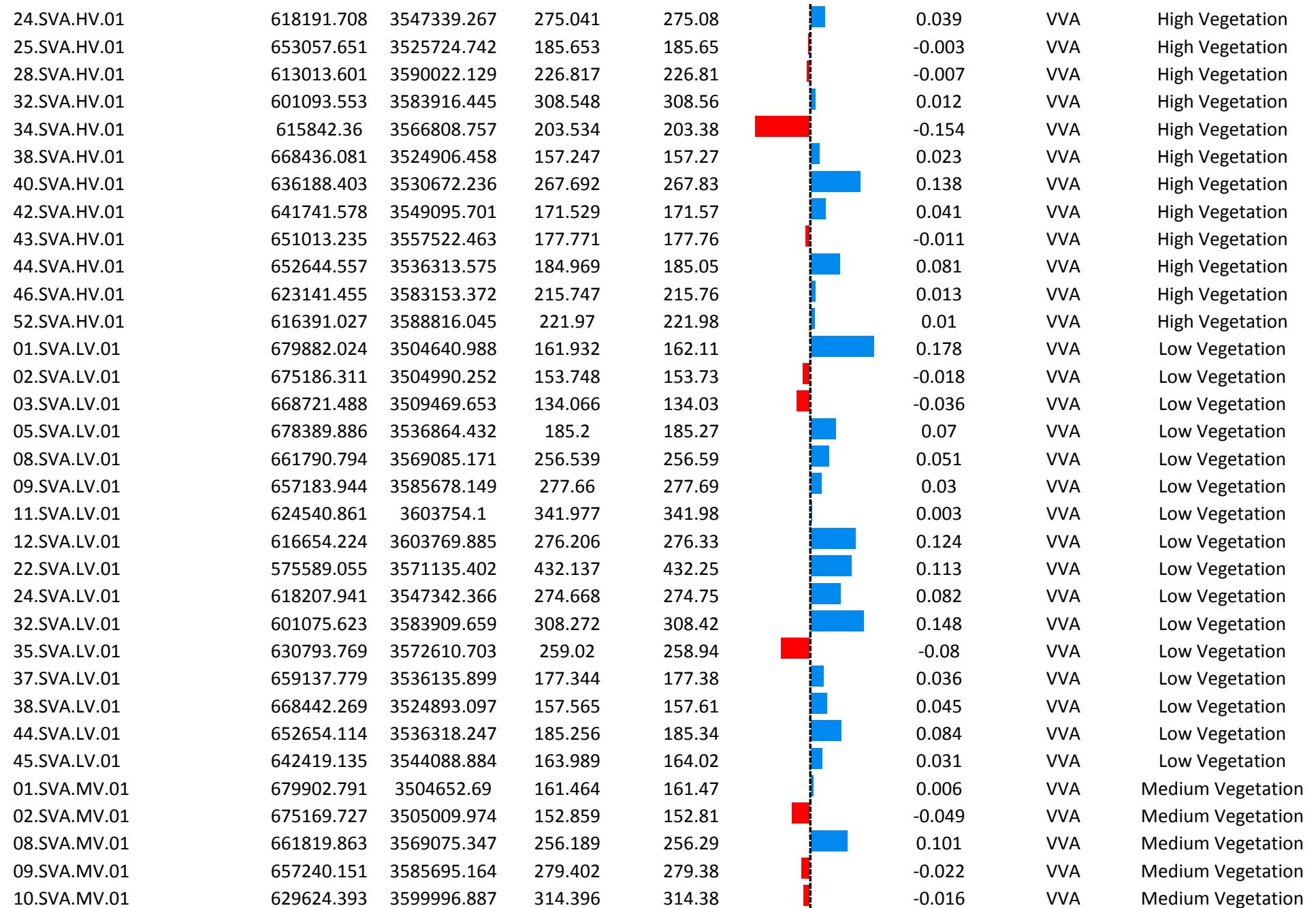
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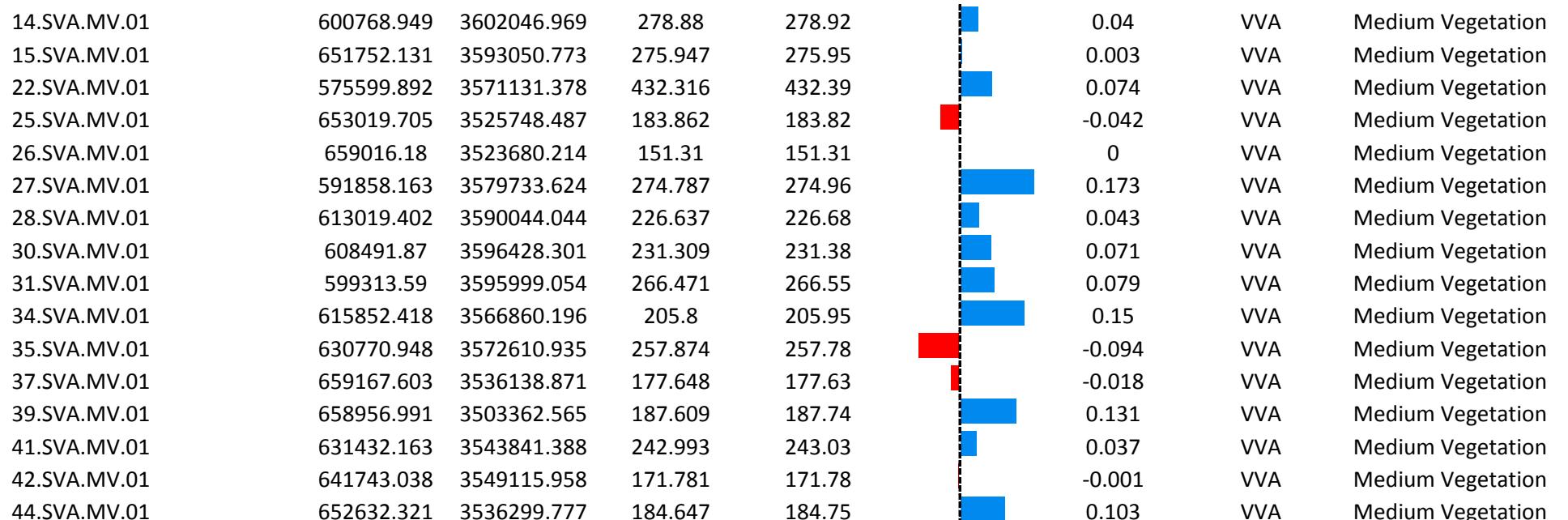
### TerraSolid LiDAR Accuracy

ID	Easting	Northing	Ortho_Z	Laser_Z	Dz_LAS	NVA/VVA	Land_Cover_Type	
01.FVA.BG.01	679896.009	3504632.39	161.347	161.35		0.003	NVA	Bare Ground
03.FVA.BG.01	668752.685	3509444.343	134.359	134.38		0.021	NVA	Bare Ground
04.FVA.BG.01	682186.691	3529138.289	216.391	216.4		0.009	NVA	Bare Ground
05.FVA.BG.01	678369.878	3536872.041	185.414	185.42		0.006	NVA	Bare Ground
08.FVA.BG.01	661762.797	3569014.87	255.551	255.57		0.019	NVA	Bare Ground
09.FVA.BG.01	657185.752	3585703.351	277.286	277.27	-0.001	-0.016	NVA	Bare Ground
11.FVA.BG.01	624563.925	3603771.825	341.716	341.71		-0.006	NVA	Bare Ground
12.FVA.BG.01	616663.064	3603761.4	276.027	276.04		0.013	NVA	Bare Ground
13.FVA.BG.01	610473.95	3603845.485	243.345	243.35		0.005	NVA	Bare Ground
14.FVA.BG.01	600798.74	3602030.409	277.992	278.08	0.009	0.088	NVA	Bare Ground
15.FVA.BG.01	651751.97	3593037.295	276.364	276.35	-0.001	-0.014	NVA	Bare Ground
16.FVA.BG.01	638509.42	3591080.313	282.122	282.09	-0.003	-0.032	NVA	Bare Ground
18.FVA.BG.01	589558.261	3598774.51	281.733	281.77	0.004	0.037	NVA	Bare Ground
20.FVA.BG.01	578237.685	3583761.845	320.929	320.93		0.001	NVA	Bare Ground
22.FVA.BG.01	575614.89	3571128.789	432.408	432.46	0.006	0.052	NVA	Bare Ground
24.FVA.BG.01	618177.42	3547284.008	273.711	273.75	0.005	0.039	NVA	Bare Ground
25.FVA.BG.01	653056.595	3525688.68	187.413	187.44	0.002	0.027	NVA	Bare Ground
26.FVA.BG.01	658995.964	3523699.39	151.337	151.36		0.003	NVA	Bare Ground
27.FVA.BG.01	591890.661	3579753.382	274.643	274.65		0.007	NVA	Bare Ground
31.FVA.BG.01	599304.309	3596002.81	266.407	266.45	0.005	0.043	NVA	Bare Ground
32.FVA.BG.01	601172.063	3583934.166	309.117	309.14	0.003	0.023	NVA	Bare Ground
34.FVA.BG.01	615853.256	3566818.966	203.917	203.92		0.003	NVA	Bare Ground
35.FVA.BG.01	630783.127	3572580.606	259.959	259.88	-0.079	-0.079	NVA	Bare Ground
38.FVA.BG.01	668455.033	3524889.652	157.794	157.8		0.006	NVA	Bare Ground
39.FVA.BG.01	658891.84	3503369.347	186.601	186.65		0.049	NVA	Bare Ground
40.FVA.BG.01	636144.385	3530694.876	267.086	267.14	0.006	0.054	NVA	Bare Ground
42.FVA.BG.01	641717.404	3549077.345	171.604	171.6	-0.004	-0.004	NVA	Bare Ground
43.FVA.BG.01	651023.124	3557521.514	177.616	177.65		0.034	NVA	Bare Ground
44.FVA.BG.01	652651.873	3536260.678	184.931	184.97		0.039	NVA	Bare Ground
46.FVA.BG.01	623158.989	3583172.255	216.012	216.03		0.018	NVA	Bare Ground
51.FVA.BG.01	647714.891	3580126.295	255.409	255.41		0.001	NVA	Bare Ground

52.FVA.BG.01	616375.926	3588773.939	221.356	221.36		0.004	NVA	Bare Ground
53.FVA.BG.01	623275.822	3582860.206	212.185	212.21		0.025	NVA	Bare Ground
04.FVA.HP.01	682189.707	3529151.556	216.391	216.38		-0.011	NVA	Hard Pavement
06.FVA.HP.01	664916.213	3538983.804	175.471	175.42		-0.051	NVA	Hard Pavement
10.FVA.HP.01	629657.098	3600045.106	315.852	315.81		-0.042	NVA	Hard Pavement
12.FVA.HP.01	616645.736	3603756.877	276.216	276.26		0.044	NVA	Hard Pavement
14.FVA.HP.01	600804.942	3602048.035	278.491	278.57		0.079	NVA	Hard Pavement
16.FVA.HP.01	638520.436	3591025.679	281.517	281.5		-0.017	NVA	Hard Pavement
18.FVA.HP.01	589540.546	3598766.22	281.803	281.83		0.027	NVA	Hard Pavement
19.FVA.HP.01	581439.025	3596571.517	346.821	346.86		0.039	NVA	Hard Pavement
21.FVA.HP.01	565023.796	3579331.352	443.197	443.22		0.023	NVA	Hard Pavement
22.FVA.HP.01	575592.933	3571145.817	432.422	432.43		0.008	NVA	Hard Pavement
24.FVA.HP.01	618192.206	3547303.871	274.285	274.28		-0.005	NVA	Hard Pavement
34.FVA.HP.01	615834.429	3566842.063	204.61	204.59		-0.02	NVA	Hard Pavement
35.FVA.HP.01	630753.284	3572649.269	255.868	255.8		-0.068	NVA	Hard Pavement
37.FVA.HP.01	659175.854	3536130.292	177.615	177.57		-0.045	NVA	Hard Pavement
39.FVA.HP.01	658934.967	3503359.847	187.274	187.31		0.036	NVA	Hard Pavement
41.FVA.HP.01	631441.618	3543830.304	243.192	243.21		0.018	NVA	Hard Pavement
43.FVA.HP.01	650993.748	3557537.244	177.836	177.86		0.024	NVA	Hard Pavement
44.FVA.HP.01	652662.347	3536260.001	185.192	185.26		0.068	NVA	Hard Pavement
52.FVA.HP.01	616396.03	3588792.172	221.13	221.1		-0.03	NVA	Hard Pavement
02.FVA.PS.01	675185.6	3504979.463	153.532	153.51		-0.022	NVA	Packed Sand
03.FVA.PS.01	668750.247	3509482.51	134.425	134.36		-0.065	NVA	Packed Sand
04.FVA.PS.01	682177.974	3529179.988	217.242	217.24		-0.002	NVA	Packed Sand
05.FVA.PS.01	678322.19	3536870.947	185.492	185.51		0.018	NVA	Packed Sand
09.FVA.PS.01	657187.646	3585660.53	277.221	277.21		-0.011	NVA	Packed Sand
10.FVA.PS.01	629640.432	3600048.527	315.097	315.12		0.023	NVA	Packed Sand
11.FVA.PS.01	624598.64	3603787.711	341.865	341.88		0.015	NVA	Packed Sand
12.FVA.PS.01	616681.15	3603765.257	275.469	275.51		0.041	NVA	Packed Sand
13.FVA.PS.01	610485.346	3603863.963	243.174	243.21		0.036	NVA	Packed Sand
14.FVA.PS.01	600769.744	3602030.755	278.441	278.48		0.039	NVA	Packed Sand
15.FVA.PS.01	651760.972	3593016.469	277.002	276.98		-0.022	NVA	Packed Sand
16.FVA.PS.01	638527.313	3591043.842	282.187	282.14		-0.047	NVA	Packed Sand







All Points

All Units in Meters

Average dz	0.017
Minimum dz	-0.154
Maximum dz	0.178
# of Observations	146
RSMEz	0.054
Std deviation	0.051

Terrasolid NVA	Meters		
# of Observations		84	
Root mean square		0.0362	3.62 cm
Std deviation		0.0351	
95% Confidence Level		0.0709	7.09 cm
Terrasolid VVA	Meters		
# of Observations		62	
Root mean square		0.0714	7.14 cm
Std deviation		0.0665	
95% Percentile		0.1475	14.75 cm

NVA By Land Type		
Bare Ground	Meters	
# of Observations		33
RSMEz		0.0325
95% Confidence Level		0.0638
Hard Pavement	Meters	
# of Observations		19
RSMEz		0.0401
95% Confidence Level		0.0786
Packed Sand	Meters	
# of Observations		32
RSMEz		0.0373
95% Confidence Level		0.0730

VVA By Land Type		
High Vegetation	Meters	
# of Observations		25
RSMEz		0.0544
95% Percentile		0.0756
Low Vegetation	Meters	
# of Observations		16
RSMEz		0.0852
95% Percentile		0.1555
Medium Vegetation	Meters	
# of Observations		21
RSMEz		0.0775
95% Percentile		0.1500

LAS	NVA	BG	HP	PS	VVA	HV	LV	MV
RSMEz	3.62 cm	3.25 cm	4.01 cm	3.73 cm	7.14 cm	5.44 cm	8.52 cm	7.75 cm
95% Confidence Level	7.09 cm	6.38 cm	7.86 cm	7.3 cm	-----	-----	-----	-----
95% Percentile	-----	-----	-----	-----	14.75 cm	7.56 cm	15.55 cm	15 cm
DEM	NVA	BG	HP	PS	VVA	HV	LV	MV
RSMEz	3.55 cm	3.37 cm	3.89 cm	3.53 cm	7.17 cm	5.13 cm	8.74 cm	7.9 cm
95% Confidence Level	6.97 cm	6.61 cm	7.63 cm	6.91 cm	-----	-----	-----	-----
95% Percentile	-----	-----	-----	-----	13.75 cm	7.96 cm	16.24 cm	12.37 cm