Lidar Mapping Report

Acquisition, Processing, and Delivery of Airborne Lidar Elevation Data for NE_Southwest_2020_D20

USGS CONTRACT: G16PC00029

CONTRACTOR: Merrick-Surdex Joint Venture, LLP (MSJV)

TASK ORDER NUMBER: 140G0220F0296

TASK ORDER NAME: NE_Southwest_2020_D20

- **PRJ_ID:** 198546
- **WU_IDs:** 198543, 220700

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Glossary of Terms

Term	Description
AGL	Above Ground Level
AGPS	Airborne Global Positioning System
AGNSS	Airborne Global Navigation Satellite System
ANPD	Aggregate Nominal Pulse Density
ANPS	Aggregate Nominal Pulse Spacing
ASPRS	American Society of Photogrammetry and Remote Sensing
AT	Aerial Triangulation
CD	Compact Disk
CMS	Certified Mapping Scientist
CORS	Continuous Operating Reference Station
CP	Certified Photogrammetrist
CRS	Coordinate Reference System
CVA	Consolidated Vertical Accuracy
DEM	Digital Elevation Model
DFIRM	Digital Flood Insurance Rate Maps
DPA	Defined Project Area
DSM	Digital Surface Model
DTM	Digital Terrain Model
DVD	Digital Versatile Disk / Digital Video Disk
DXF	Data Exchange Format / Drawing Interchange
FIRM	Flood Insurance Rate Maps
FEMA	Federal Emergency Management
FGDC	Federal Geographic Data Committee
FVA	Fundamental Vertical Accuracy
FY	Fiscal Year
GIS	Geographic Information System
GISP	Geographic Information System Professional
GNSS	Global Navigation Satellite System
GPS	Global Positioning System
GSD	Ground Sample Distance
HARN	High Accuracy Reference Network
HDD	Hard Drive Disk
HPGN	High Precision Geodetic Network
IMU	Inertial Measurement Unit
INS	Inertial Navigation System
LAS	(or .las) – industry accepted LIDAR data exchange file format
LB	License Business
LS	Land Surveyor
LiDAR	(or Lidar) Light Detection And Ranging
MARS®	Merrick Advanced Remote Sensing
MSJV	Merrick-Surdex Joint Venture, LLP
MSL	Mean Sea Level
NAD	North American Datum
NDEP	National Digital Elevation Program
NGP	National Geospatial Program
NGS	National Geodetic Survey
NMAS	National Map Accuracy Standards

No.	Number
NPS	Nominal Point Spacing
NSRS	National Spatial Reference System
NSSDA	National Standard for Spatial Data
NVA	Non-vegetated Vertical Accuracy
OPUS	Online Positioning User Service
PDOP	Positional Dilution Of Precision
PLS	Professional Land Surveyor
PLSS	Public Land Survey System
ppsm	Points (or pulses) per square meter
PSM	Professional Surveyor and Mapper
QL2	Quality Level One
QL2	Quality Level Two
RLS	Registered Land Surveyor
RGB	Red, Green, Blue (i.e., three-band image)
RGBNIR	Red, Green, Blue, Near Infra-Red (i.e., four-band image)
RMSE	Root Mean Square Error
SBET	Smoothed Best Estimated Trajectory
SHA	Secured Hash Standard
SPCS	State Plane Coordinate System
SVA	Supplemental Vertical Accuracy
TIN	Triangular Irregular Network
USGS	United States Geological Survey
VVA	Vegetated Vertical Accuracy
WP_ID	Work Package ID (USGS)
WU_ID	Work Unit ID (USGS)
XML	Extensible Markup Language

Project Summary

MSJV was awarded the NE_Southwest_2020_D20 Task Order by the United States Geologic Survey (USGS) to provide high resolution data set of QL2 lidar of approximately 12,596.56 square miles including the full and partial Nebraska counties of: Banner, Chase, Dundy, Garden, Hayes, Hitchcock, Keith, Lincoln, Morrill, Perkins, Sioux and Scotts Bluff. This project will support the National Resources Conservation Service (NRCS) high resolution elevation enterprise program, USGS, and the 3DEP mission.

The lidar mapping requirements and deliverables meet Quality Level One (QL2) standards for final deliverables as outlined in the *USGS-NGP Lidar Base Specification 2020, Revision A* (https://www.usgs.gov/3DEP/lidarspec). QL2 lidar specifications suggest a pulse density of greater than or equal to two pulses per square meter (≥2ppsm) Aggregate Nominal Pulse Density (ANPD), and pulse spacing of less than or equal to seventy-one centimeters (≤0.71m) Aggregate Nominal Pulse Spacing (ANPS).

The vertical accuracy requirements of the lidar data meets or exceeds the following:

Absolute Vertical Accuracy
- ≤10cm RMSEz
- ≤19.6cm Non-vegetated Vertical Accuracy (NVA) at the 95% confidence level
- ≤30cm Vegetated Vertical Accuracy (VVA) at the 95% percentile

Relative Vertical Accuracy
- ≤6cm within individual swaths (smooth surface repeatability)
- ≤8cm $RMSD_Z$ within swath overlap (between adjacent swaths)

Task Order CRS (Coordinate Reference System)
- Projection – Universal Transverse Mercator (UTM), Zone 13 North (13N) and Zone 14 North (14N)
- Horizontal Datum - North American Datum of 1983 (NAD 83), National Adjustment of 2011 (NA2011) (epoch 2010.00)
- Vertical Datum – North American Vertical Datum of 1988 (NAVD 88); using the latest NGS-approved geoid (i.e., **GEOID18**) for converting ellipsoid heights to orthometric elevations
- Horizontal Units – Meters
- Vertical Units – Meters
- EPSG Codes
  - UTM Zone 13N = EPSG 6342
  - UTM Zone 14N = EPSG 6343

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The contents of this report summarize the methods used to calibrate and classify the lidar data as well as the results of these methods for the NE_Southwest_2020_D20 Task Order, otherwise known as PRJ_ID: 198546. Results of this report are given for the delineated WU_IDs: 198543 and 220700 (representing UTM Zones 13N and 14N respectively).

Lidar Flight Information

The acquisition area or Defined Project Area (DPA) for the NE_Southwest_2020_D20 Task Order is delineated by the extent of the client approved Esri shapefile (*NE_Southwest_2020_D20_100mBUFFER*). MSJV acquired the QL2 lidar point cloud utilizing Optech Galaxy PRIME lidar sensors. The project was flown and then processed and delivered in blocks (Work Units). Each work unit represents a different UTM zone.
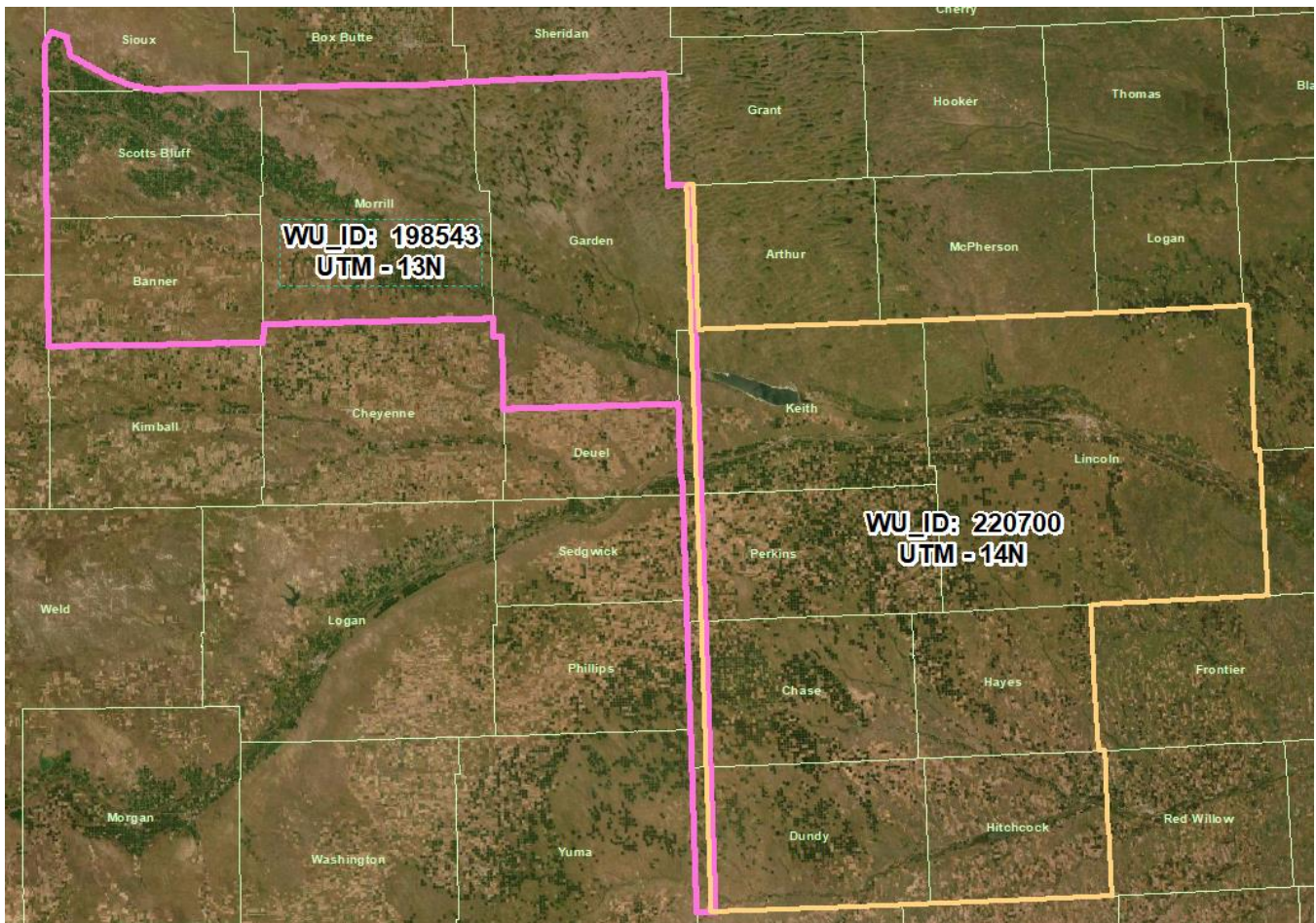
Aerial Mission(s)

Lidar acquisition was collected using fixed wing aircraft and Optech Galaxy PRIME lidar sensors staging from a variety of airports around the project area. Up to eight return values are recorded for each pulse which ensures the greatest chance of ground returns in a heavily forested area. Lidar data collection was accomplished between November 21, 2020 and March 5, 2021. Each mission represents a lift of the aircraft and system from the ground, collects data, and lands again. Multiple lifts within a day are represented by Mission A, B, C, and D.

The table below relates each mission to the date collected, the sensor and serial number used, and the actual average MSL in meters.

Mission(s)	Date	Sensor S/N	Actual Avg. MSL (m)
201121_A	November 21, 2020	5060380	4000
201123_A	November 23, 2020	5060380	3900
201125_A	November 25, 2020	5060380	3970
201126_A	November 26, 2020	5060380	3850
201127_A	November 27, 2020	5060380	3730
201128_A	November 28, 2020	5060380	3720
201129_A	November 29, 2020	5060380	3775
201203_A	December 3, 2020	5060380	3845
201204_A	December 4, 2020	5060380	3865
201204_A	December 4, 2020	5060428	3797
201205_A	December 5, 2020	5060380	3900
201205_A	December 5, 2020	5060428	3717
201206_A	December 6, 2020	5060380	3950
201206_A	December 6, 2020	5060428	3710
201206_B	December 6, 2020	5060428	3749
201206_C	December 6, 2020	5060428	3744
201206_D	December 6, 2020	5060428	3765
201206_E	December 6, 2020	5060428	3745
201207_A	December 7, 2020	5060380	3960
201207_A	December 7, 2020	5060428	3715
201207_B	December 7, 2020	5060428	3723
201207_C	December 7, 2020	5060428	3747
201207_D	December 7, 2020	5060428	3763
201208_A	December 8, 2020	5060380	4090
201208_A	December 8, 2020	5060428	3789
201213_A	December 13, 2020	5060428	3704
201213_B	December 13, 2020	5060428	3821
201219_A	December 19, 2020	5060428	3830
201220_A	December 20, 2020	5060428	3829
201221_A	December 21, 2020	5060428	3836
201221_B	December 21, 2020	5060428	3870
201222_A	December 22, 2020	5060428	3851
201222_B	December 22, 2020	5060428	3870
201226_A	December 26, 2020	5060428	3816
210301_A	March 1, 2021	5060428	3717
210302_A	March 2, 2021	5060428	3849

210303_A	March 3, 2021	5060428	3860
210303_B	March 3, 2021	5060428	3851
210304_A	March 4, 2021	5060428	3871
210305_A	March 5, 2021	5060428	3871
210305_B	March 5, 2021	5060428	3876

GNSS / IMU Data

A five-minute IMU initialization is conducted on the ground, with the aircraft engines running, prior to flight, to establish fine alignment of the IMU. In air IMU calibration maneuvers were performed at the beginning and ending of all mission collections to ensure the best forward and reverse trajectory processing using the highest quality IMU calibration. During the data collection, the operator recorded information on log sheets which includes weather conditions, lidar operation parameters, and flight line statistics. Data is sent back to the main office for preliminary processing to check overall quality of GNSS / IMU data and to ensure sufficient overlap between flight lines. Any problematic data may be reflown immediately as required.

The airborne GNSS data was post-processed using Applanix POSPac Mobile Mapping Suite version 8.x. A fixed-bias carrier phase solution was computed in both the forward and reverse chronological directions. Whenever practical, lidar acquisition was limited to periods when the PDOP was less than 4.0. PDOP indicates satellite geometry relating to position. Generally, PDOP's of 3.0 or less result in a good quality solution, however PDOP's between 3.0 and 5.0 can still yield good results most of the time. PDOP's over 6.0 are of questionable results and PDOP's of over 7.0 usually result in a poor solution. Usually as the number of satellites increase the PDOP decreases. Other quality control checks used for the GNSS include analyzing the combined separation of the forward and reverse GNSS processing from one CORS station and the results of the combined separation when processed from two different CORS stations. An analysis of the number of satellites, present during the flight and data collection times, is also performed.

The GNSS trajectory was combined with the raw IMU data and post-processed using POSPac Mobile Mapping Suite version 8.x. The SBET and refined attitude data are then utilized in the Optech LMS lidar processing software to compute the laser point-positions. The trajectory is combined with the laser range measurements to produce lidar point cloud data.

POS reports for each mission are included on the delivery media: ..\metadata\reports\Lidar_Report\POS_reports

GNSS Controls

Virtual Ground GNSS Base Station(s) were used to control the lidar airborne flight lines. Post processed Trimble CenterPoint® RTX™ correction service is a high-accuracy, satellite-delivered global positioning service. This technology provides high accuracy GNSS positioning without the use of traditional reference station based differential RTK infrastructure and delivers very high cm level accuracy. In addition, CORS are at times used to further QC or enhance the airborne GNSS solution.

Acquisition Data Check

Validation of field data is a time-critical process. Since re-mobilizations have significant financial and schedule impacts, the JV's goal for every project is to ensure that all data has been completely and accurately acquired before leaving the project site. While coverage is one aspect to verify, the JV focuses on checking aspects that prove adherence to all lidar base specification requirements as well as a full data integrity check as well. Using the MARS® QC Module, the following tests are performed on each mission:

Test	Methodology	Purpose
Returns	Tabular stats and graphics	To ensure all return collecting system components are working properly.
Intensity	Tabular stats and graphics	To ensure all intensity collecting system components are working properly. Also, to look for potential, but rare, laser return path misalignment system issues.
Density	Density calculations by swath but also by spot location, binary raster, density raster, project aggregate, and Voronoi density reporting	To ensure the minimum required lidar point density is achieved for every flight line.
Data Void	Binary raster method as required by LBS	To ensure no unallowable data voids are present
Spatial Distribution	Binary raster method as required by LBS	To ensure all swaths have been collected with the appropriate spatial distribution requirement
Relative Accuracy	Flightline separation raster	An initial look at interswath accuracy, prior to full calibration, to ensure there are no severe and unexpected calibration issues
Sensor Calibration	Scan direction 1 vs 2 separation raster and channel to channel separation raster if applicable	An initial look at intraswath accuracy, prior to full calibration, to ensure there are no severe and unexpected calibration issues
Flight Line Coverage	Coverage rasters	To ensure full coverage of the project boundary. This is a second but different look for data voids.
Sensor Anomalies	Shaded relief raster	To ensure there are no sensor anomalies visible in a shaded relief raster

Lidar Calibration – see appendix 1 for a more detailed workflow description

MSJV takes great care to ensure all lidar acquisition missions are carried out in a manner conducive to post-processing an accurate dataset. Proper Airborne GNSS surveying techniques are always followed including pre- and post-mission static initializations. In-air IMU alignments (figure-eights) are performed both before and after on-site collection to ensure proper calibration of the IMU accelerometers and gyros.

A minimum of one cross-flight is planned throughout the project area across all flightlines and over roadways where possible. The cross-flight provides a common control surface used to remove any vertical discrepancies in the lidar data between flightlines. The cross-flight is critical to ensure flightline ties across the project area. The areas of overlap between flightlines are used to boresight (calibrate) the lidar point cloud to achieve proper flightline to flightline alignment in all three axes. Each lidar mission flown is accompanied by a hands-on boresight in the office.

MSJV understands that high accuracy/quality data cannot be generated from black-boxed-processed lidar data. Many parts of the downstream process suffer from poorly calibrated lidar data. We have a proven process that produces data that meets relative and absolute accuracy specifications reserved for QL0 data for all quality level products. Our all-encompassing lidar calibration process includes the following steps:

1. Sensor model calibration (scale, edge curl, range offsets, etc.)
2. Application of timing offsets (POS and scanner)
3. Calibrating scan direction 0 versus scan direction 1 (inbound versus outbound if applicable)
4. Channel to channel calibration (if applicable)
5. IMU to scanner misalignment angles (heading, pitch, roll deltas) calibration
6. Final geometric calibration tweaks including:
    a. easting, northing, elevations shifts
    b. heading, roll, pitch shifts
    c. easting, northing, elevations drifts

d. heading, roll, pitch drifts

e. fluctuating elevation

Below is an example of before (left) and after (right) flightline separation rasters having been through this highly effective process. The remaining non-green colors are areas of steep terrain.
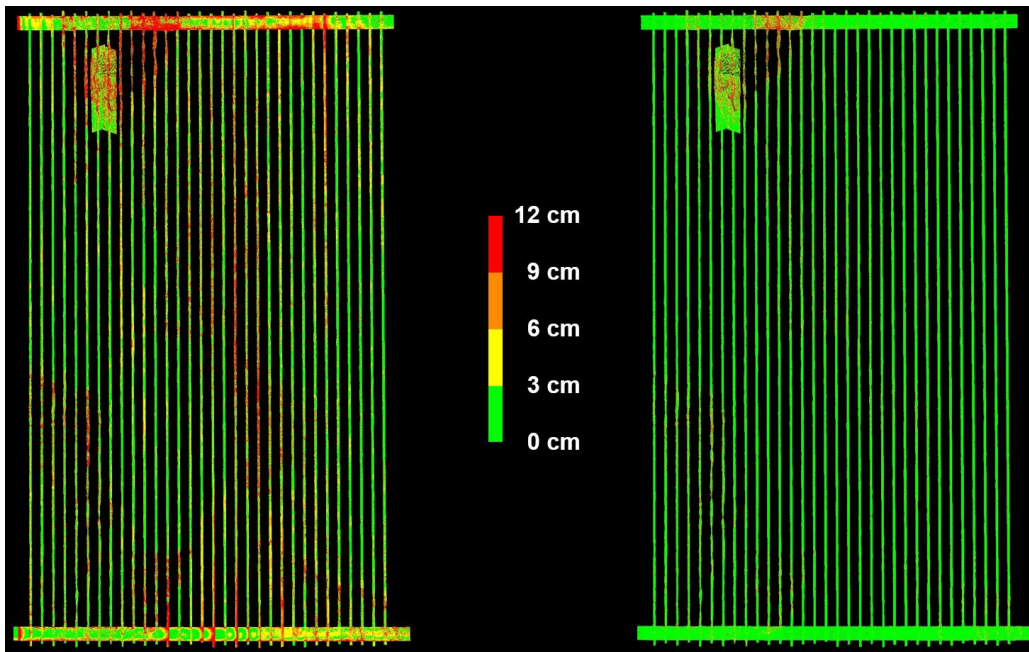
Project wide results are equally as accurate.
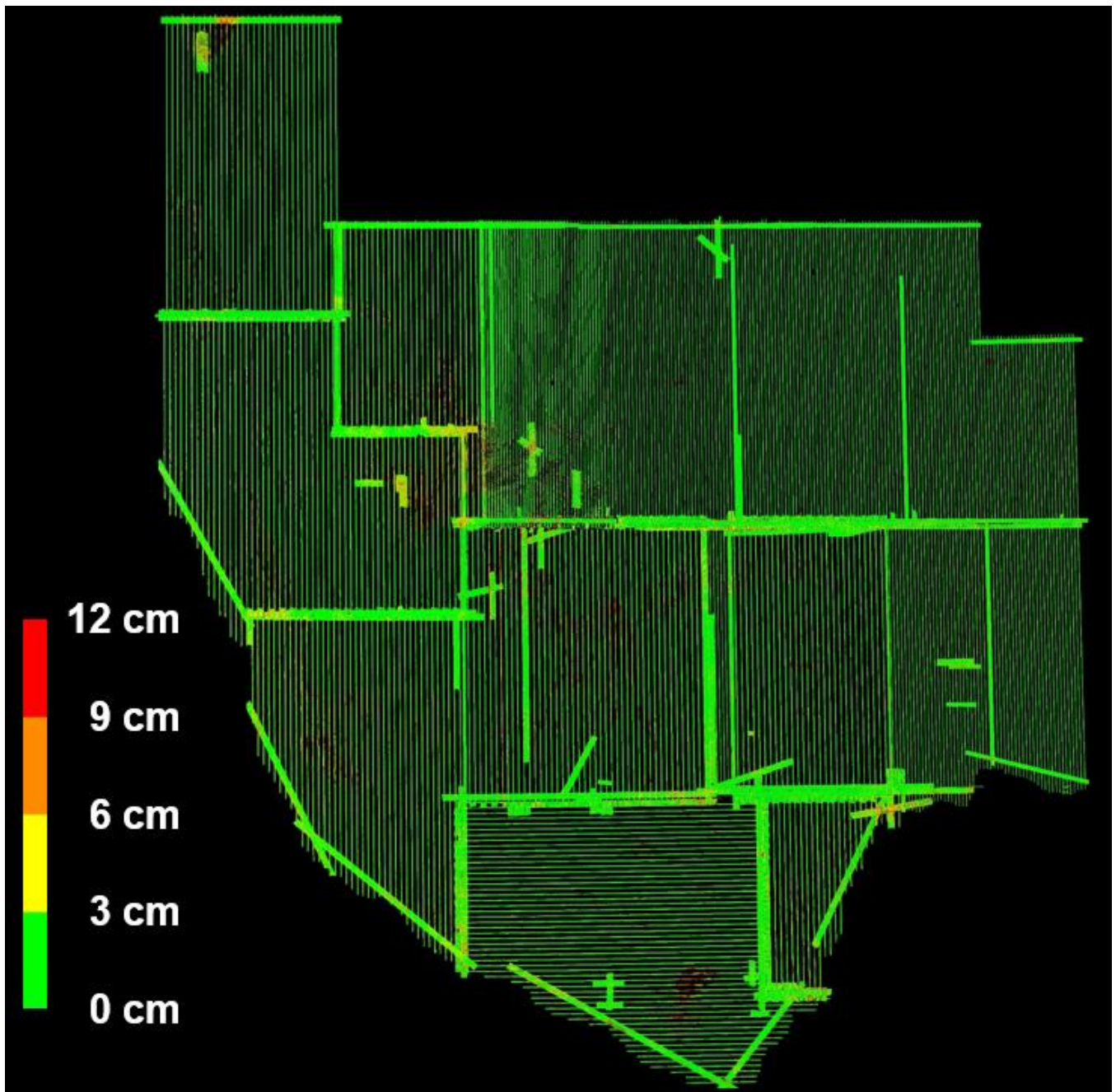
After boresighting is complete a detailed statistical report is generated to check relative and absolute accuracies before filtering of lidar begins. The calibration process yields excellent absolute accuracies, as can be seen for this example project.

Relative Accuracy – flight line to flight line

The project representative flight line separation raster (below) depicts the vertical separation of flight lines by thematically coloring the separation magnitude on a color ramp based on relative distance.

WU_ID: 198543
UTM - 13N

WU_ID: 220700
UTM - 14N

| 0 | 0.080 | 0.160 | 0.240 | 0.800 (Meter) |
| 0 | 0.262 | 0.525 | 0.787 | 2.625 (Feet) |

Unfiltered Lidar Control Point Report

The following statistical results of the lidar data compared to the lidar control points post-calibration. The results show the difference between the lidar points and the 141 surveyed ground points located in WU_IDs: 198543, 220700.

WU_ID: 198543

Project Data Unit: Meter
Vertical Accuracy Class tested: 10.0-cm
Elevation Calculation Method: Interpolated from TIN
LiDAR Classifications Included: 0-255

Check Points in Report: 56
Check Points with LiDAR Coverage: 56
Check Points (NVA): 56
Check Points (VVA): 0
Average Vertical Error Reported: -0.005 Meter
Maximum (highest) Vertical Error Reported: 0.101 Meter
Median Vertical Error Reported: -0.014 Meter
Minimum (lowest) Vertical Error Reported: -0.083 Meter
Standard deviation of Vertical Error: 0.049 Meter
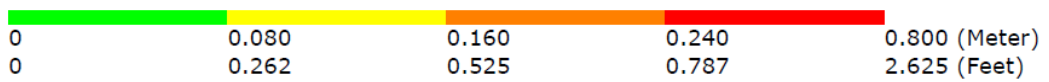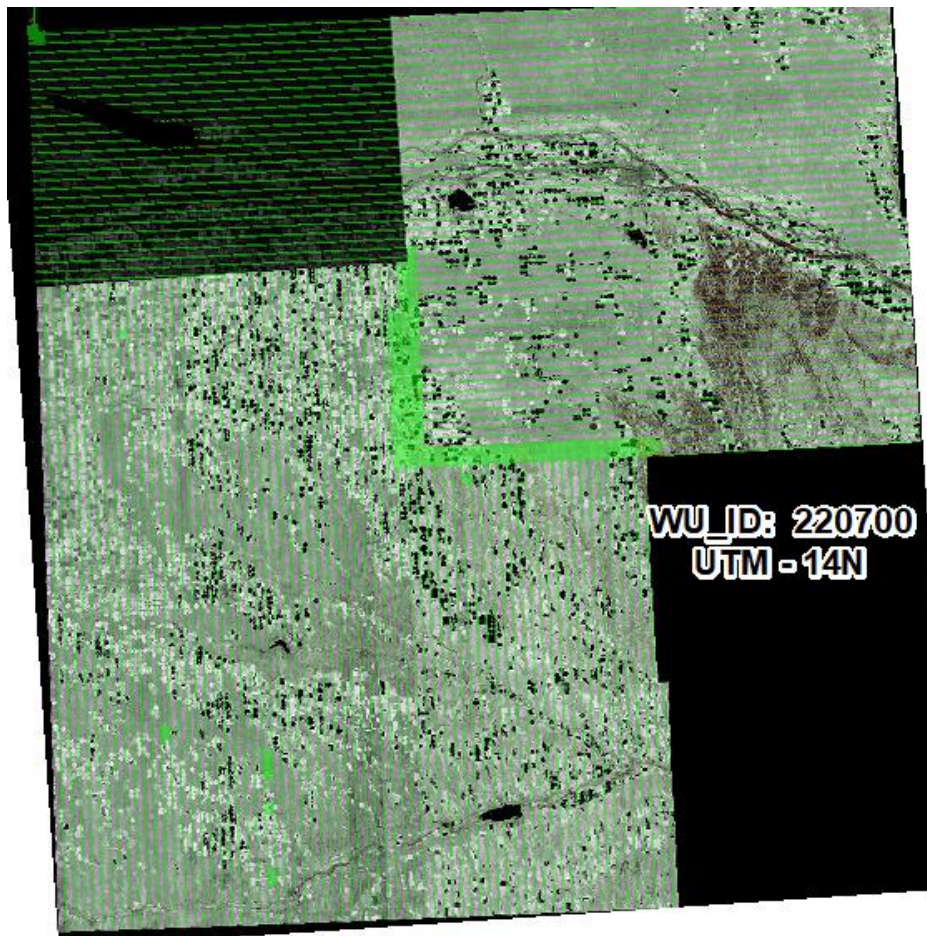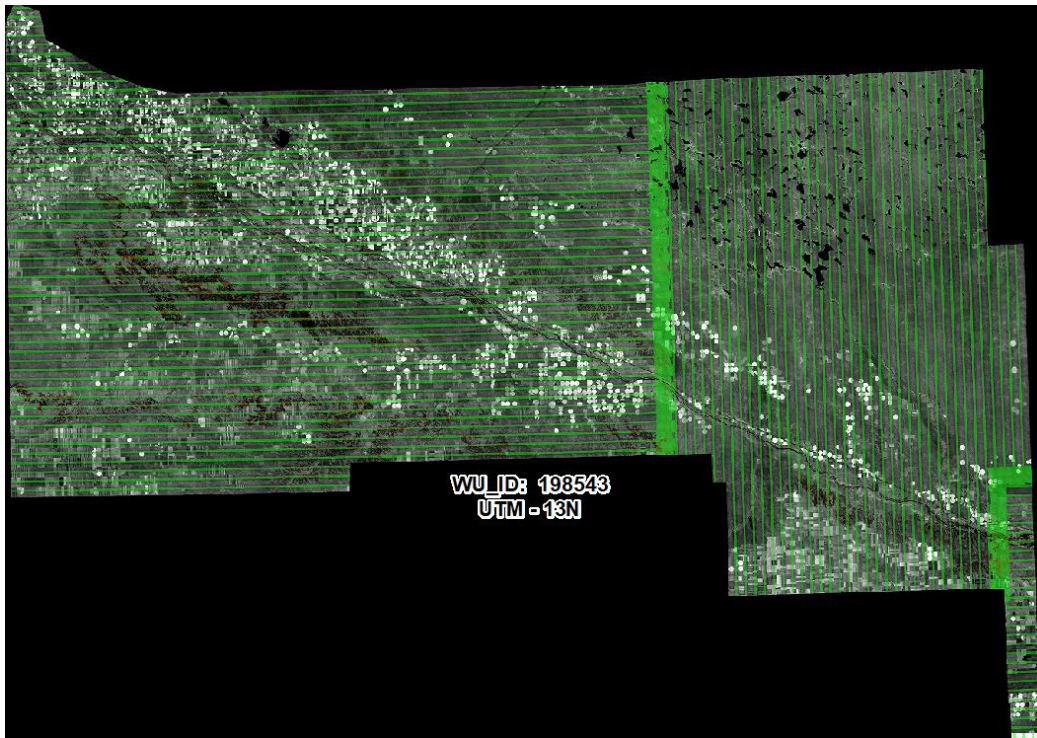Skewness of Vertical Error: 0.656
Kurtosis of Vertical Error: -0.570
Non-vegetated Vertical Accuracy (NVA) RMSE(z): 4.838cm PASS
Non-vegetated Vertical Accuracy (NVA) at the 95% Confidence Level +/-: 9.482cm PASS
FGDC/NSSDA Vertical Accuracy at the 95% Confidence Level +/-: 9.482cm
Non-vegetated Vertical Accuracy (NVA) RMSE(z) (DEM): 5.213cm PASS
Non-vegetated Vertical Accuracy (NVA) at the 95% Confidence Level +/- (DEM): 10.217cm PASS

This data set was tested to meet ASPRS Positional Accuracy Standard for Digital Geospatial Data (2014) for a 10.0-cm RMSEz Vertical Accuracy Class. Actual NVA accuracy was found to be RMSEz = 4.838cm, equating to +/- 9.482cm at the 95% confidence level.

WU_ID: 220700

Project Data Unit: Meter
Vertical Accuracy Class tested: 10.0-cm
Elevation Calculation Method: Interpolated from TIN
LiDAR Classifications Included: 0-255

Check Points in Report: 85
Check Points with LiDAR Coverage: 85
Check Points (NVA): 85
Check Points (VVA): 0
Average Vertical Error Reported: 0.014 Meter
Maximum (highest) Vertical Error Reported: 0.152 Meter
Median Vertical Error Reported: 0.020 Meter
Minimum (lowest) Vertical Error Reported: -0.183 Meter
Standard deviation of Vertical Error: 0.052 Meter
Skewness of Vertical Error: -0.622
Kurtosis of Vertical Error: 2.247
Non-vegetated Vertical Accuracy (NVA) RMSE(z): 5.320cm PASS
Non-vegetated Vertical Accuracy (NVA) at the 95% Confidence Level +/-: 10.427cm PASS
FGDC/NSSDA Vertical Accuracy at the 95% Confidence Level +/-: 10.427cm
Non-vegetated Vertical Accuracy (NVA) RMSE(z) (DEM): 5.230cm PASS
Non-vegetated Vertical Accuracy (NVA) at the 95% Confidence Level +/- (DEM): 10.251cm PASS

This data set was tested to meet ASPRS Positional Accuracy Standard for Digital Geospatial Data (2014) for a 10.0-cm RMSEz Vertical Accuracy Class. Actual NVA accuracy was found to be RMSEz = 5.320cm, equating to +/- 10.427cm at the 95% confidence level.

Lidar Control Point Layout

Lidar Filtering and Classification

The lidar filtering process encompasses a series of automated and manual steps to classify the boresighted point cloud data set. Each project represents unique characteristics in terms of cultural features (urbanized vs. rural areas), terrain type and vegetation coverage. These characteristics are thoroughly evaluated at the onset of the project to ensure that the appropriate automated filters are applied and that subsequent manual filtering yields correctly classified data. Data is most often classified by ground and "unclassified", but specific project applications can include a wide variety of classifications including but not limited to buildings, vegetation, power lines, etc. A variety of software packages are used for the auto-filtering, manual filtering and QC of the classified data.

MSJV used the ASPRS LAS Specification Version 1.4 – R15 (ASPRS, 2011, published 09 July 2019), Point Data Record Format 6 for this project and classified the lidar point cloud in accordance with the following classification classes and bitflags. The following outlines project specific requirements.

- Class 1 = Unclassified
- Class 2 = Bare-earth Ground
- Class 7 = Low point (noise)
- Class 9 = Water
- Class 17 = Bridge decks
- Class 18 = High noise

- Class 20 = Ignored Ground (breakline proximity)
- Class 21 = Snow (if present and identifiable)
- Class 22 = Temporal exclusion (typically non-favored data in intertidal zones)

- Bitflags
  - <u>Overlap:</u> Any part of a swath that also is covered by any part of any other swath.
  - <u>Withheld:</u> Within the LAS file specification, a single bit flag indicating that the associated lidar point is geometrically anomalous or unreliable and should be ignored for all normal processes.

MSJV has developed several customized automated filters that are applied to the lidar data set based on project specifications, terrain, and vegetation characteristics. A filtering macro, which may contain one or more filtering algorithms, is executed to derive LAS files separated into the different classification groups as defined in the ASPRS classification table. The macros are tested in several portions of the project area to verify the appropriateness of the filters. Often, there is a combination of several filter macros that optimize the filtering based on the unique characteristics of the project. Automatic filtering generally yields a ground surface that is 85-90% valid, so additional editing (hand-filtering) is required to produce an accurate ground surface.

Lidar data is next taken into a graphic environment using MARS® to manually re-classify (or hand-filter) "noise" and other features that may remain in the ground classification after auto filter. A cross-section of the post auto-filtered surface is viewed to assist in the reclassification of non-ground data artifacts. The following is an example of re-classification of the non-ground points (elevated features) that need to be excluded from the true ground surface. Certain features such as berms, hilltops, cliffs and other features may have been aggressively auto-filtered and points will need to be re-classified into the ground classification. Data in the profile view displays non-ground (Unclassified, class 1) in grey and ground in brown/tan (Class 2). In **Figure 1**, a small building was not auto-filtered and needs to be manually re-classified. Note that **Figure 2** has the building points reclassified to unclassified from the true ground surface.
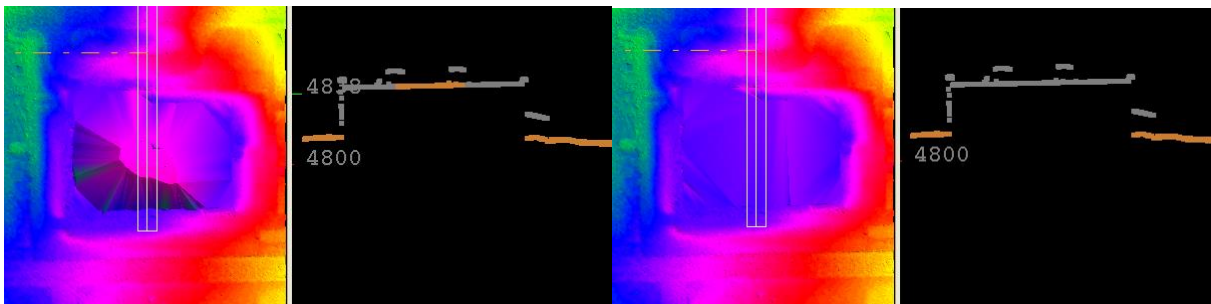

| Figure 1 | Figure 2 |

A combination of automated and semi-automated routines to classify buildings and vegetation. We expect that the classified buildings will meet expected filtering criteria.

At this point, individual lidar points from the original point cloud have now been parsed into separate classifications.

Filtered Lidar Checkpoint Report

After hand-filtering has been completed and quality checked, a Checkpoint Report is generated to validate that the accuracy of the ground surface is within the defined accuracy specifications. Each surveyed ground check point is compared to the lidar surface by interpolating an elevation from a Triangulated Irregular Network (TIN)

of the surface. The MARS® derived report provides an in-depth statistical report, including an RMSE of the vertical errors; a primary component in most accuracy standards and a statistically valid assessment of the overall accuracy of the ground surface.

The below lidar check point reports provide statistics for 463 ground survey checkpoints (268 NVA, 195 VVA) used to validate the final filtered lidar surface.

WU_ID: 198543

Units: Meter (/Feet)

Vertical Accuracy Class tested: 10-cm

Check Points in defined project area (DPA):	186
Check Points with Lidar Coverage	186
Check Points with Lidar Coverage (NVA)	106
Check Points with Lidar Coverage (VVA)	80
Average Z Error (NVA)	-0.010/-0.033
Maximum Z Error (NVA)	0.212/0.694
Median Z Error (NVA)	-0.019/-0.062
Minimum Z Error (NVA)	-0.111/-0.364
Standard deviation of Vertical Error (NVA)	0.052/0.170
Skewness of Vertical Error (NVA)	1.172
Kurtosis of Vertical Error (NVA)	2.715
Non-vegetated Vertical Accuracy (NVA) RMSE(z) [1]	0.053/0.172 PASS
Non-vegetated Vertical Accuracy (NVA) at the 95% Confidence Level +/- [1]	0.103/0.338 PASS
FGDC/NSSDA Vertical Accuracy at the 95% Confidence Level +/-	0.103/0.338
Non-vegetated Vertical Accuracy (NVA) RMSE(z) (DEM) [2]	0.051/0.168 PASS
Non-vegetated Vertical Accuracy (NVA) at the 95% Confidence Level (DEM) +/- [2]	0.101/0.330 PASS
Vegetated Vertical Accuracy (VVA) at the 95th Percentile (TIN) +/- [1]	0.189/0.621 PASS
Vegetated Vertical Accuracy (VVA) at the 95th Percentile (DEM) +/- [2]	0.184/0.604 PASS

This data set was tested to meet ASPRS Positional Accuracy Standard for Digital Geospatial Data (2014) for a 10-cm RMSEz Vertical Accuracy Class. Actual NVA accuracy was found to be RMSEz = 5.3cm, equating to +/- 10.3cm at the 95% confidence level. Actual VVA accuracy was found to be +/- 18.4cm at the 95th percentile.

[1] This value is calculated from TIN-based testing of the lidar point cloud data.

[2] This value is calculated from RAM-based grid testing of the lidar data. The grid cells are sized according to the Quality Level selected, and are defined in the USGS NGP Lidar Base Specification Version 2.1 (Table 6).

Units: Meter (/Feet)

Vertical Accuracy Class tested: 10-cm

|	
Check Points in defined project area (DPA):	277
Check Points with Lidar Coverage	277
Check Points with Lidar Coverage (NVA)	162
Check Points with Lidar Coverage (VVA)	115
Average Z Error (NVA)	0.014/0.045
Maximum Z Error (NVA)	0.229/0.751
Median Z Error (NVA)	0.016/0.052
Minimum Z Error (NVA)	-0.262/-0.861
Standard deviation of Vertical Error (NVA)	0.066/0.215
Skewness of Vertical Error (NVA)	-0.932
Kurtosis of Vertical Error (NVA)	3.938
Non-vegetated Vertical Accuracy (NVA) RMSE(z) [1]	0.067/0.219 PASS
Non-vegetated Vertical Accuracy (NVA) at the 95% Confidence Level +/- [1]	0.131/0.429 PASS
FGDC/NSSDA Vertical Accuracy at the 95% Confidence Level +/-	0.131/0.429
Non-vegetated Vertical Accuracy (NVA) RMSE(z) (DEM) [2]	0.067/0.220 PASS
Non-vegetated Vertical Accuracy (NVA) at the 95% Confidence Level (DEM) +/- [2]	0.131/0.431 PASS
Vegetated Vertical Accuracy (VVA) at the 95th Percentile (TIN) +/- [1]	0.248/0.813 PASS
Vegetated Vertical Accuracy (VVA) at the 95th Percentile (DEM) +/- [2]	0.249/0.818 PASS

This data set was tested to meet ASPRS Positional Accuracy Standard for Digital Geospatial Data (2014) for a 10-cm RMSEz Vertical Accuracy Class. Actual NVA accuracy was found to be RMSEz = 6.7cm, equating to +/- 13.1cm at the 95% confidence level. Actual VVA accuracy was found to be +/- 24.9cm at the 95th percentile.

[1] This value is calculated from TIN-based testing of the lidar point cloud data.

[2] This value is calculated from RAM-based grid testing of the lidar data. The grid cells are sized according to the Quality Level selected, and are defined in the USGS NGP Lidar Base Specification Version 2.1 (Table 6).

Lidar Checkpoint Layout
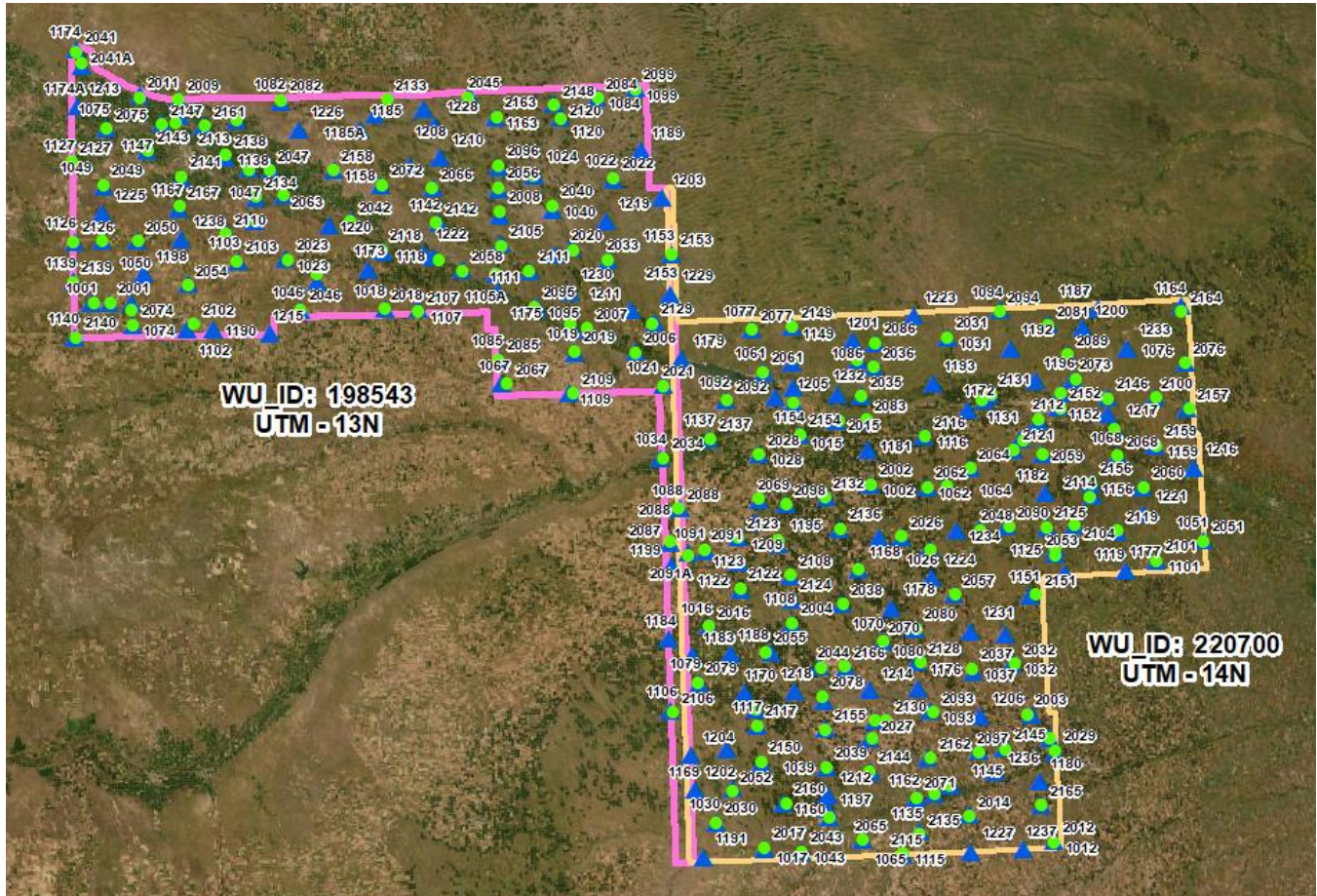
Hydro-flattening Breakline Collection

Hydro- flattening breaklines are captured per the *USGS-NGP Lidar Base Specification 2020, Revision A*. Final hydro-flattened breaklines features are appropriately turned into polygons (flat elevations) and polylines (decreasing by elevation) and are used to reclassify ground points in water to water (Class 9). The lidar points around the breaklines are reclassified to ignored ground (Class 10) based on the planned collected point density.

The next step in the process is the hydro-flattening breakline collection required for the development of the hydro-flattened DEMs. MSJV will capture hydro-flattening breaklines for waterbodies greater than or equal to approximately eight-tenths (~0.8) hectare (e.g., ~100-meter diameter); double-sided streams and rivers that are greater than or equal to thirty-meters (≥30m) in (nominal) width, and; any visible islands greater than or equal to approximately four-tenths (~0.4) hectare. Criteria for *Non-Tidal Boundary Waters* and *Tidal Waters* are assumed not applicable. No single-line streams or drainages will be collected, nor will any planimetric features that could be utilized as traditional breaklines. All downstream hydro-flattening breaklines require monotonicity (e.g., streams and rivers). Closed polygonal boundaries of water will maintain a fixed (i.e., flat) elevation.

Linear hydrographic features

To collect hydrographic features, MSJV uses a methodology that directly interacts with the lidar bare-earth data to collect drainage breaklines. To determine the alignment of a drainageway, the technician first views the area

as a TIN of bare-earth points using a color ramp to depict varying elevations. In areas of extremely flat terrain, the technician may need to determine the direction of flow based on measuring lidar bare-earth points at each end of the drain. The operator will then use the color ramped TIN to digitize the drainage in 2D with the elevation being attributed directly from the bare-earth LAS data. MARS® software has the capability of "flipping" views between the elevation TIN, intensity, and imagery, as necessary, to further assist in the determination of the drainage. All drainage breaklines are collected in a downhill direction. For each point collected, the software uses an user specified search radius to identify the lowest point within that proximity. Within each radius, if a bare-earth point is not found that is lower than the previous point, the elevation for subsequent point remains the same as the previous point. This forces the drain to always flow in a downhill direction. Waterbodies that are embedded along a drainageway are validated to ensure consistency with the downhill direction of flow.

This methodology may differ from those of other vendors in that MSJV relies on the bare-earth data to attribute breakline elevations. As a result of our methodology, there is no mismatch between lidar bare-earth data and breaklines that might otherwise be collected in stereo 3D as a separate process. This is particularly important in densely vegetated areas where breaklines collected in 3D from imagery will most likely not match (either horizontally or vertically), the more reliable lidar bare-earth data.

MSJV has the capability of "draping" 2D breaklines to a bare-earth elevation model to attribute the "z" as opposed to the forced downhill attribution methodology described above. However, the problem with this process is the "pooling "effect or depressions along the drainageway caused by a lack of consistent penetration in densely vegetated areas.

Criteria of linear hydrographic breaklines are as follows:

- Linear hydrographic features (e.g., visible streams, rivers, shorelines, canals, etc.) greater than or equal to 30m wide (nominal width) will be captured as a double-lined polygon
    - linear hydrographic features must be flat and level bank-to-bank (perpendicular to the apparent flow centerline) with gradient following the immediately surrounding terrain
    - water surface edge must be at or just below the immediately surrounding terrain
    - streams should break at road crossings (e.g., culverts), and streams and rivers should not break at bridges

Waterbodies

Waterbodies are digitized from the color ramped TIN/Intensity, similar to the process described above. The elevation attribute is determined as the technician collects the hydro feature by using the lowest bare-earth point within a search radius of the polygon line being drawn.

Criteria of waterbody breaklines are as follows:

- Waterbodies (e.g., lakes, ponds, reservoirs) greater than or equal to approximately 0.8 hectares in size are surrounded by a water breakline (i.e., closed polygon)
    - waterbodies must be flat and level with a single elevation for every bank vertex
    - water surface edge must be at or just below the immediately surrounding terrain
    - long impoundments, such as reservoirs or inlets, whose water surface elevations drop when moving downstream should be treated as rivers

Color cycles provide a clear indication of where breaklines are to be collected, especially hydrographic breaklines. **Figure 3** demonstrates no breaklines, where **Figure 4** is breakline enforced displayed using color cycles within the MARS® software environment.
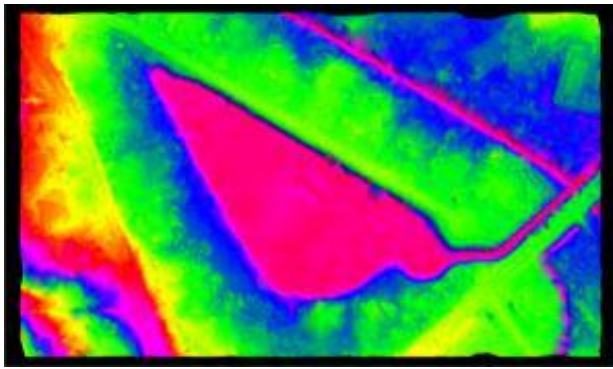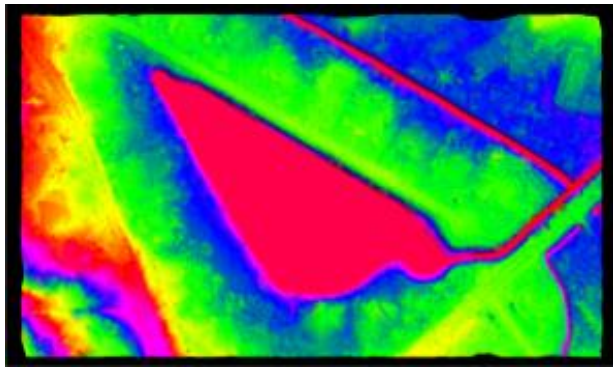
| Figure 3 | Figure 4 |

Bare-earth Digital Elevation Model (DEM)

MSJV will export the hydro-flattened classified ground (i.e., Class 2) lidar points to a **one (1m)** cell size, 32-bit floating point raster images using MARS®. The DEMs are exported to the project tiling scheme**,** and in some cases, project- or area-wide. Projection information is applied that reflects the project CRS. Culverts will not be removed from the DEMs. Bridges will be removed from the DEMs.

Intensity Images

MSJV will export all lidar points to a **one (1m)** cell size 8-bit raster image using MARS®. The intensity images are exported to the project tiling scheme. Projection information is applied that reflects the project CRS.

Contour Lines

Two Foot Contours are generated using MARS® proprietary software at the desired contour interval. Topology QC checks are completed to ensure topography is logical and complete. Additional QC checks for dangles and appropriate attribution are also verified to comply with project requirements before delivery to the client.

List of Deliverables

❖ Classified lidar point cloud
  ➢ Fully compliant ASPRS LAS 1.4-R15, point record format 6
  ➢ By tile
  ➢ Intensity values normalized (rescaled) to 16-bit
❖ Bare-earth DEM
  ➢ 1m cell size 32-bit floating point raster in Cloud Optimized GeoTIFF (COG) format (.tif)
  ➢ By tile, by county, and by project area
    ▪ Countywide and areawide mosaics in ERDAS IMG format
❖ First Return Digital Surface Model (DSM)
  ➢ 1m cell size 32-bit floating point raster in ERDAS IMG format
  ➢ By tile and by county
❖ Hillshades
  ➢ 1m cell size in ERDAS IMG format
  ➢ By county
❖ Hydro-flattened breaklines
  ➢ Project-wide Esri feature class(es) for insertion into file geodatabase
❖ Intensity Images
  ➢ 1m cell size 8-bit, 256 color gray scale in Cloud Optimized GeoTIFF (COG) format (.tif)
  ➢ By tile

- ❖ Two-foot (2') contours
  - ➢ Esri feature class(es) for insertion into file geodatabase (GDB)
  - ➢ By tile and by county
- ❖ FGDC-compliant metadata in XML format
  - ➢ LAS
  - ➢ DEM
  - ➢ DSM
  - ➢ Hillshade
  - ➢ Breaklines
  - ➢ Intensity
  - ➢ Contours
- ❖ Vertical Accuracy (Esri shapefiles)
  - ➢ Calibration (control)
  - ➢ NVA/VVA (checkpoints)
- ❖ Esri shapefiles
  - ➢ Flight index
    - ▪ Esri feature class(es) for insertion into file geodatabase (GDB)
  - ➢ 1,000m x 1,000m tiles (clipped)
  - ➢ DPA
- ❖ MARS® QC folder
  - ➢ PDF QC reports
  - ➢ Miscellaneous files
- ❖ Maximum Surface Height Raster (MSHR)
  - ➢ 1m cell size 32-bit floating point raster in Cloud Optimized GeoTIFF (COG) format (.tif)
  - ➢ By tile
- ❖ Swath Separation Image (SSI)
  - ➢ Two-meter (2m) cell size 8-bit unsigned, 3-band raster in in Cloud Optimized GeoTIFF format (.tif)
  - ➢ Area-wide mosaic(s) as 2m cell size 8-bit unsigned, 3-band raster in jpeg2000 (JP2) format
- ❖ Lidar and Mapping Report
  - ➢ Acquisition
  - ➢ Processing
  - ➢ Accuracy assessment
  - ➢ POS reports in PDF format
- ❖ Ground Control Survey Report
  - ➢ Collection
  - ➢ Processing
  - ➢ Coordinate listing (all points)
  - ➢ Photos (all points)
  - ➢ Shapefiles of coordinates (all points)

Following is a more detailed lidar calibration workflow description.

LIDAR CALIBRATION AND BLOCK LAS OUTPUT

Note: All figures represented on the following pages are for general illustration purposes, and are not
   examples derived from the project.

Initial Processing

Lidar data is output as LAS point data using Optech's Lidar Mapping Suite (LMS). LMS matches ground and roof
planes plus roof lines to self-calibrate and correct system biases. These biases occur within the hardware of the
laser scanning systems, within the Inertial Measurement Unit (IMU) and because of environmental conditions
which affect the refraction of light. The systemic biases that are corrected for include scale, roll, pitch, and
heading.

In addition to the self-calibration mode LMS runs a "production" mode which applies the self-calibration
parameters and then analyzes each individual flight line and applies small adjustments to each line to tie
overlapping lidar points even more tightly together.

Boresight Self-Calibration Processing Procedures

An LMS boresight calibration is performed on an as-needed basis to correct scale, roll, pitch and heading biases.
A minimum of three overlapping flights are flown in opposing directions with one cross flight.
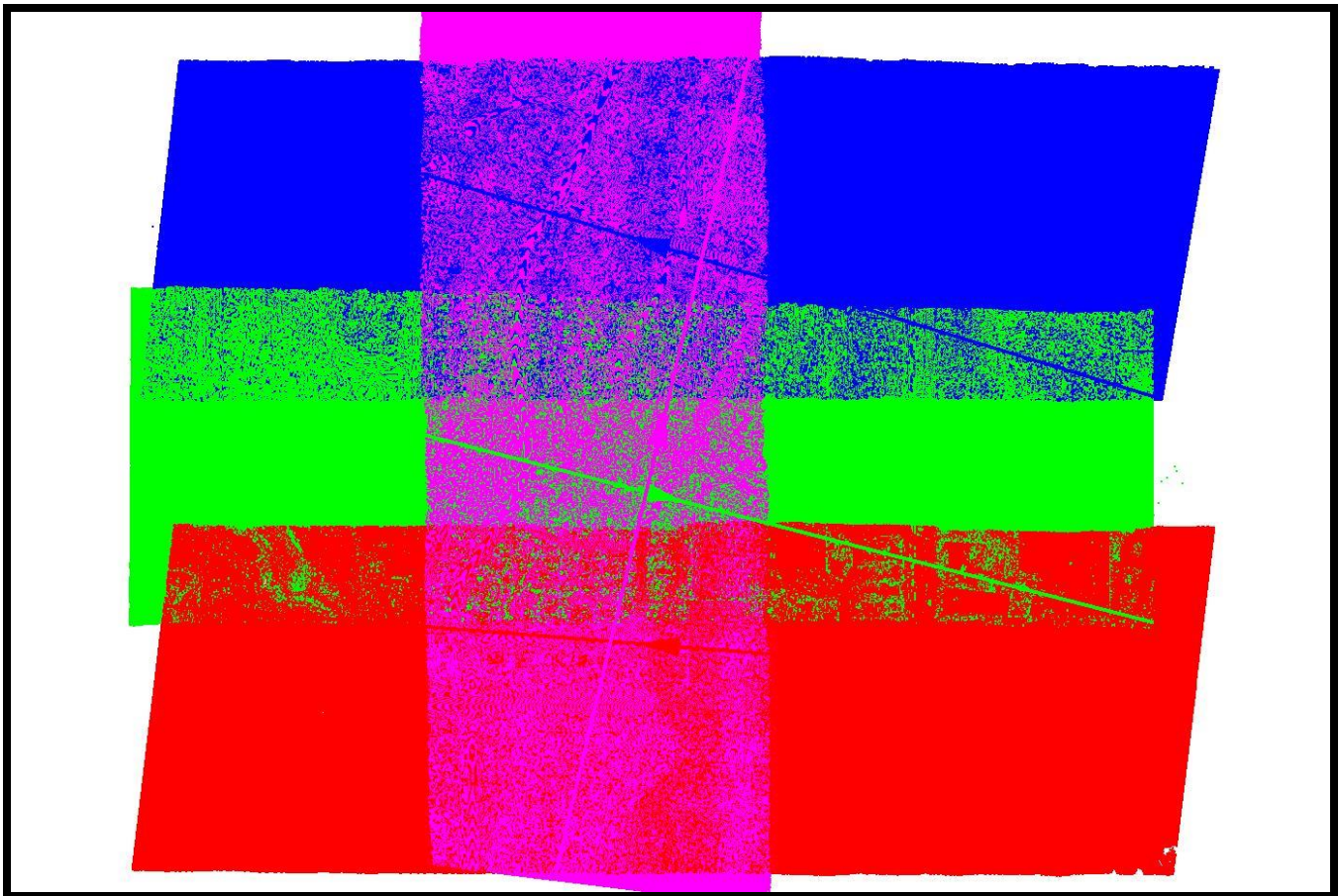
The Boresighting module frees scan angle scale, scan angle lag, XYZ boresight corrections and elevation position corrections while locking scan angle offset and XY position corrections.

The picked calibration site will have a good distribution of buildings for the self-calibration software to match ground planes, roof planes and roof lines.
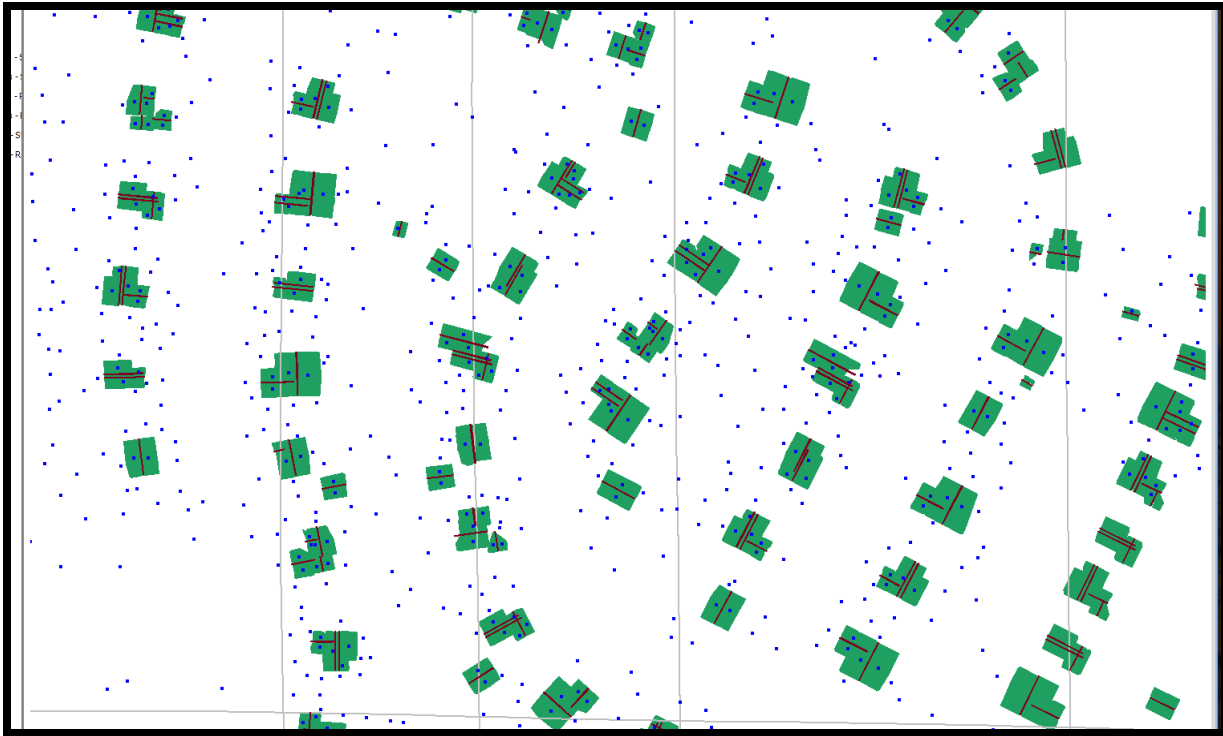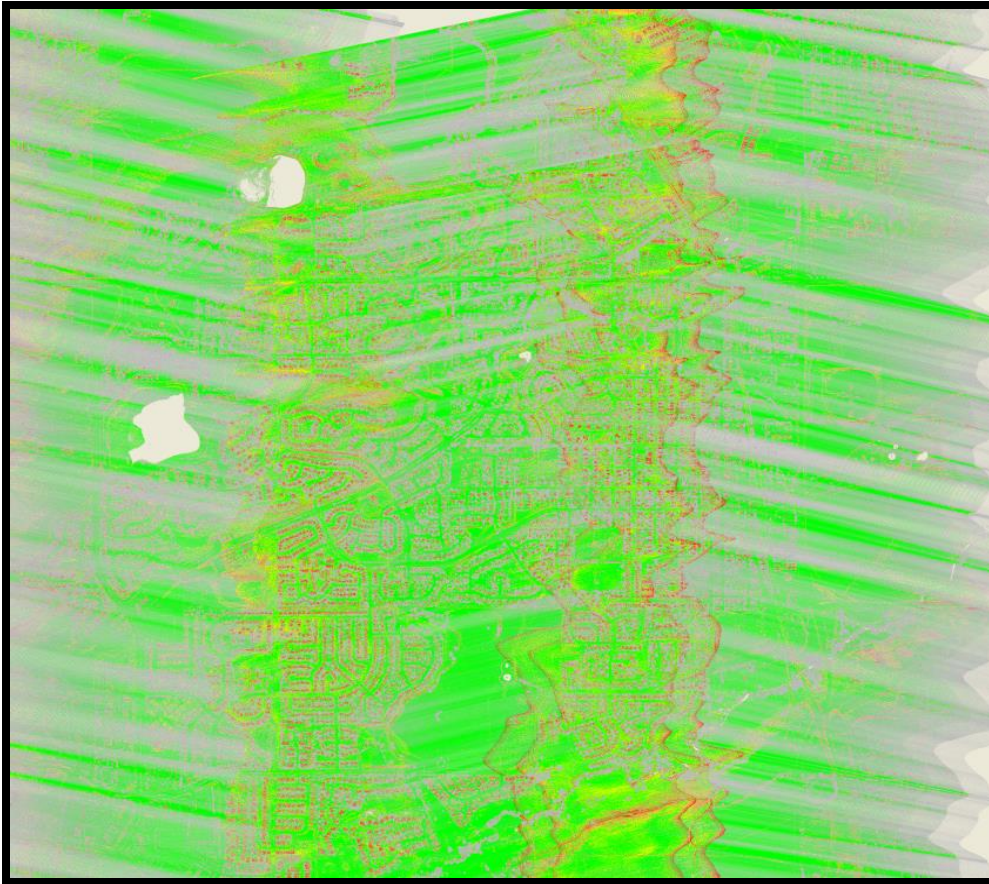

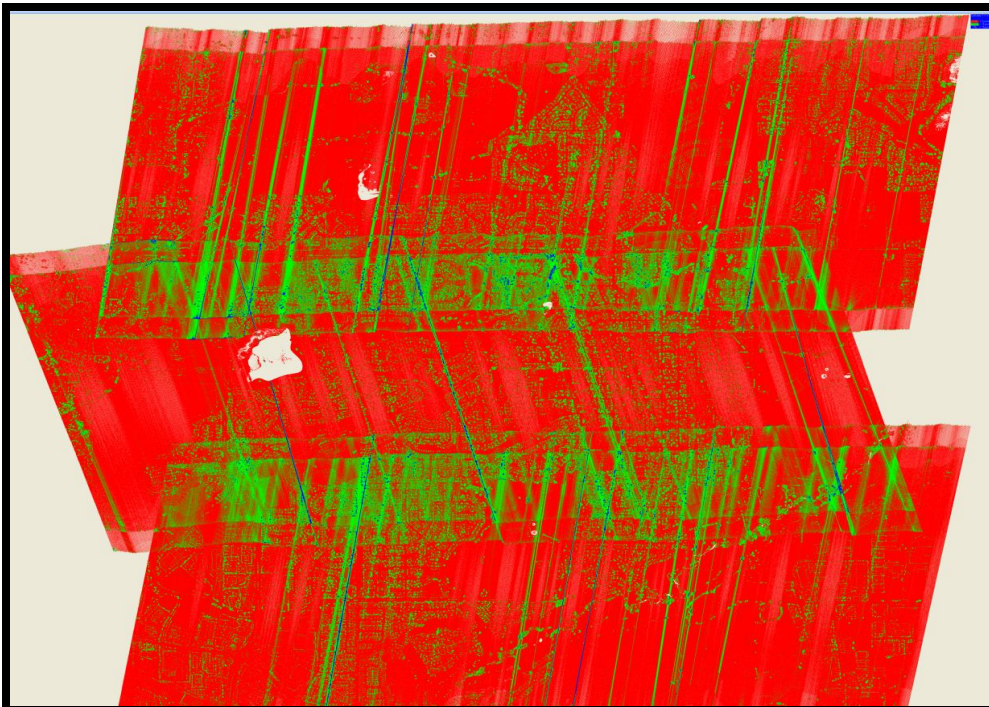At the conclusion of the self-calibration run the data is quality checked with LMS plots

Plot of plane vertical distances from datum plane.
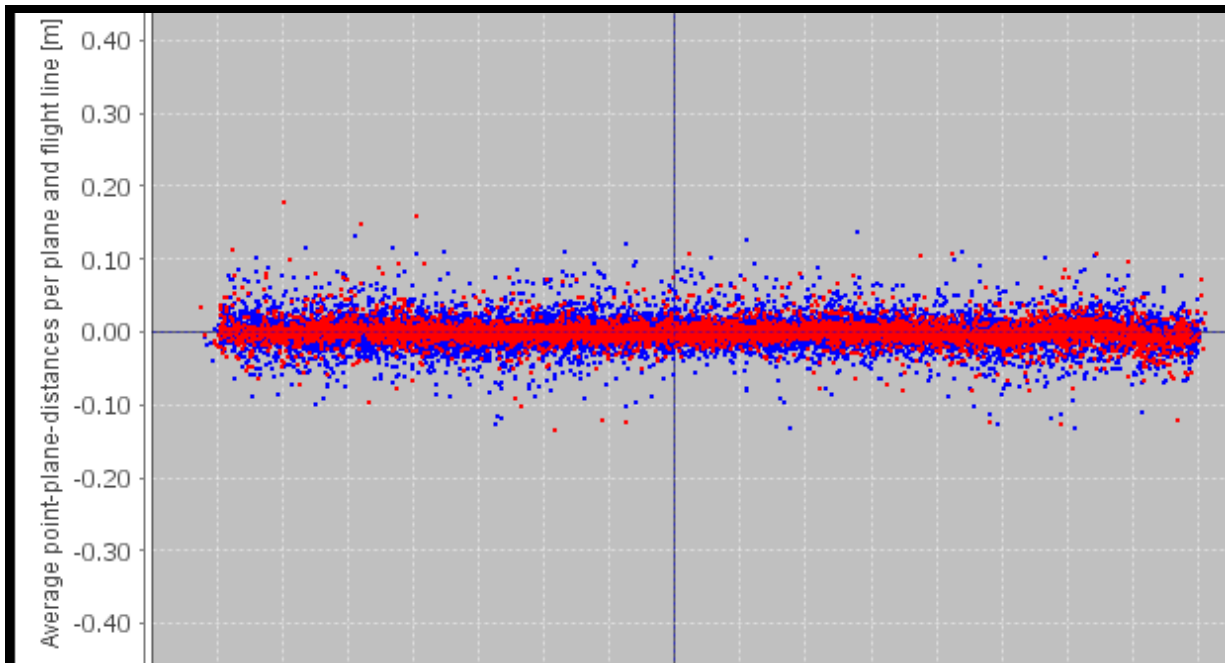
Plot of height differenced between flight lines. (Green=less than 5cm).

Plot of point densities. (Red=5-9 points per cell, green 10+ points per cell).

A Flight Line Separation Raster image is generated in Merrick Advanced Remote Sensing Software (MARS®), in this example ground returns from multiple flight lines that are fitting within 3 centimeters are colored green.
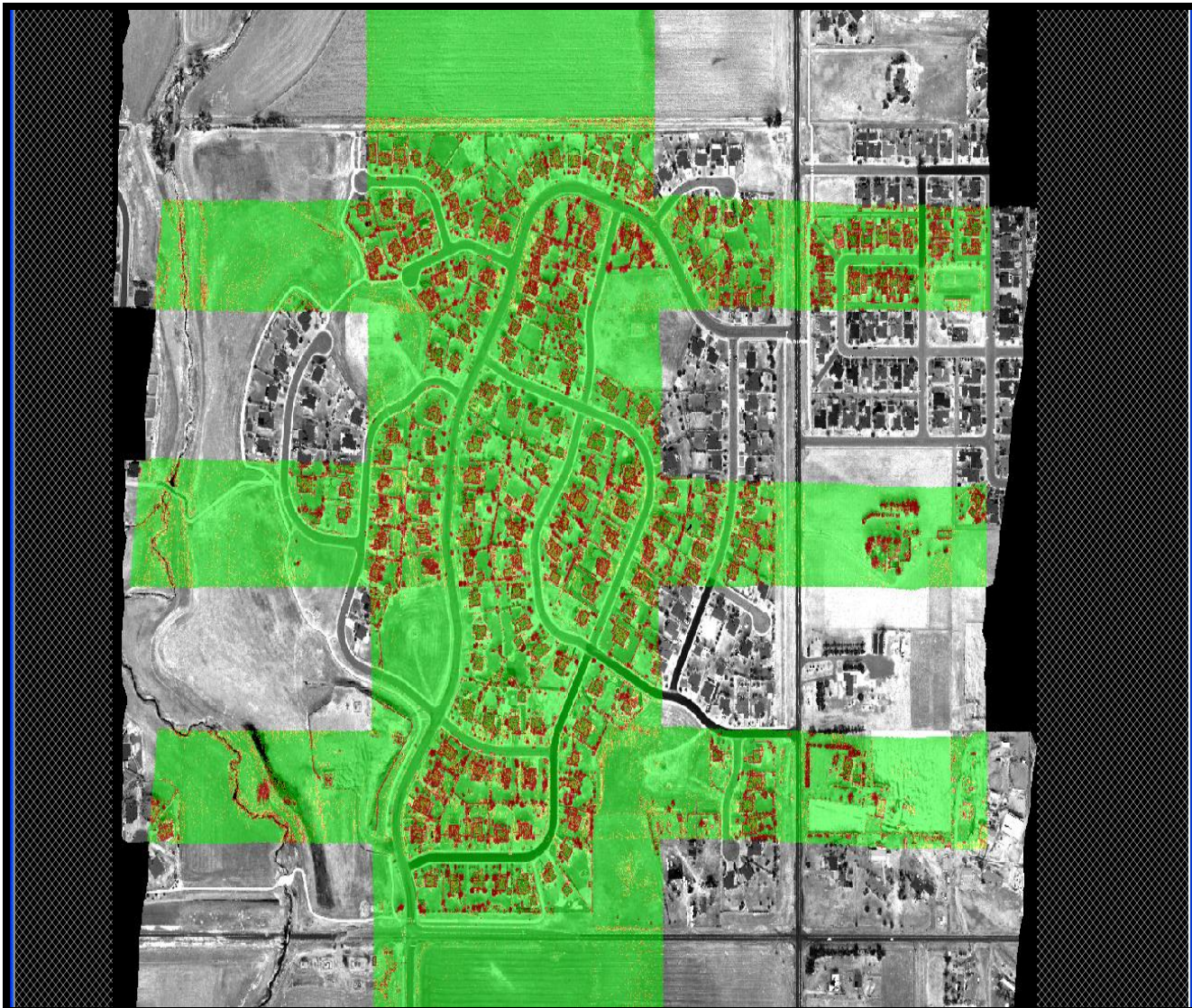
MARS® tests for internal relative vertical accuracy using inbound and outbound scan values. Again, Green is showing inbound and outbound scan data fitting to 3 centimeters.
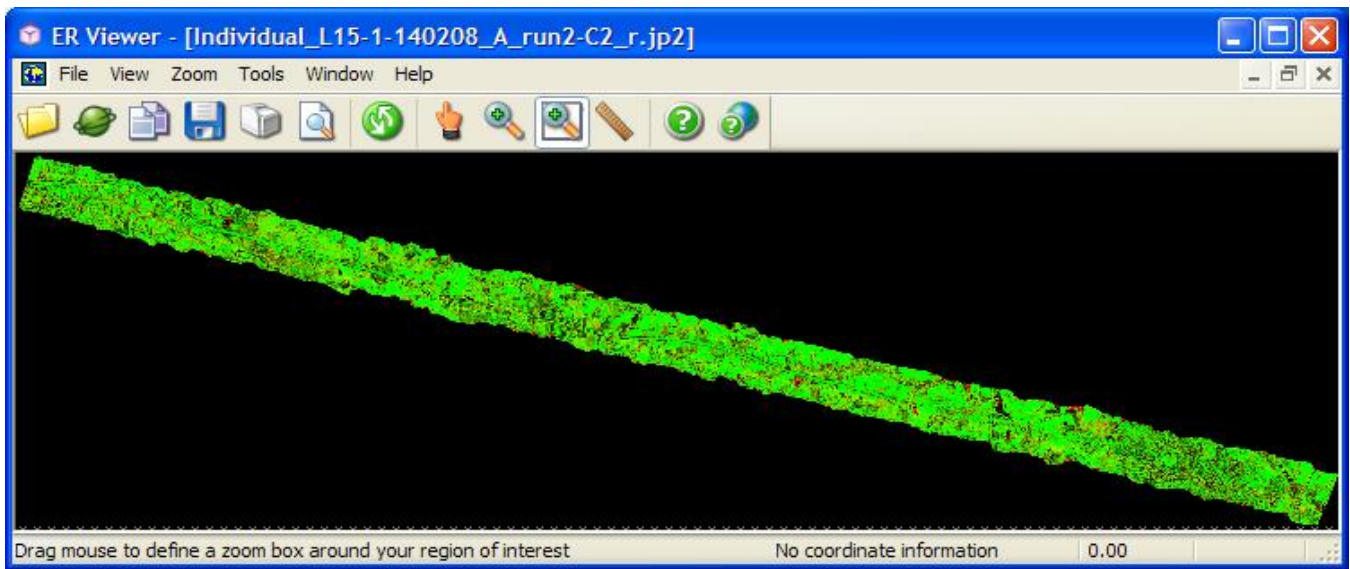
Building cross sections are checked for good alignment. Pitch and heading are checked on roof planes parallel to the flight direction.
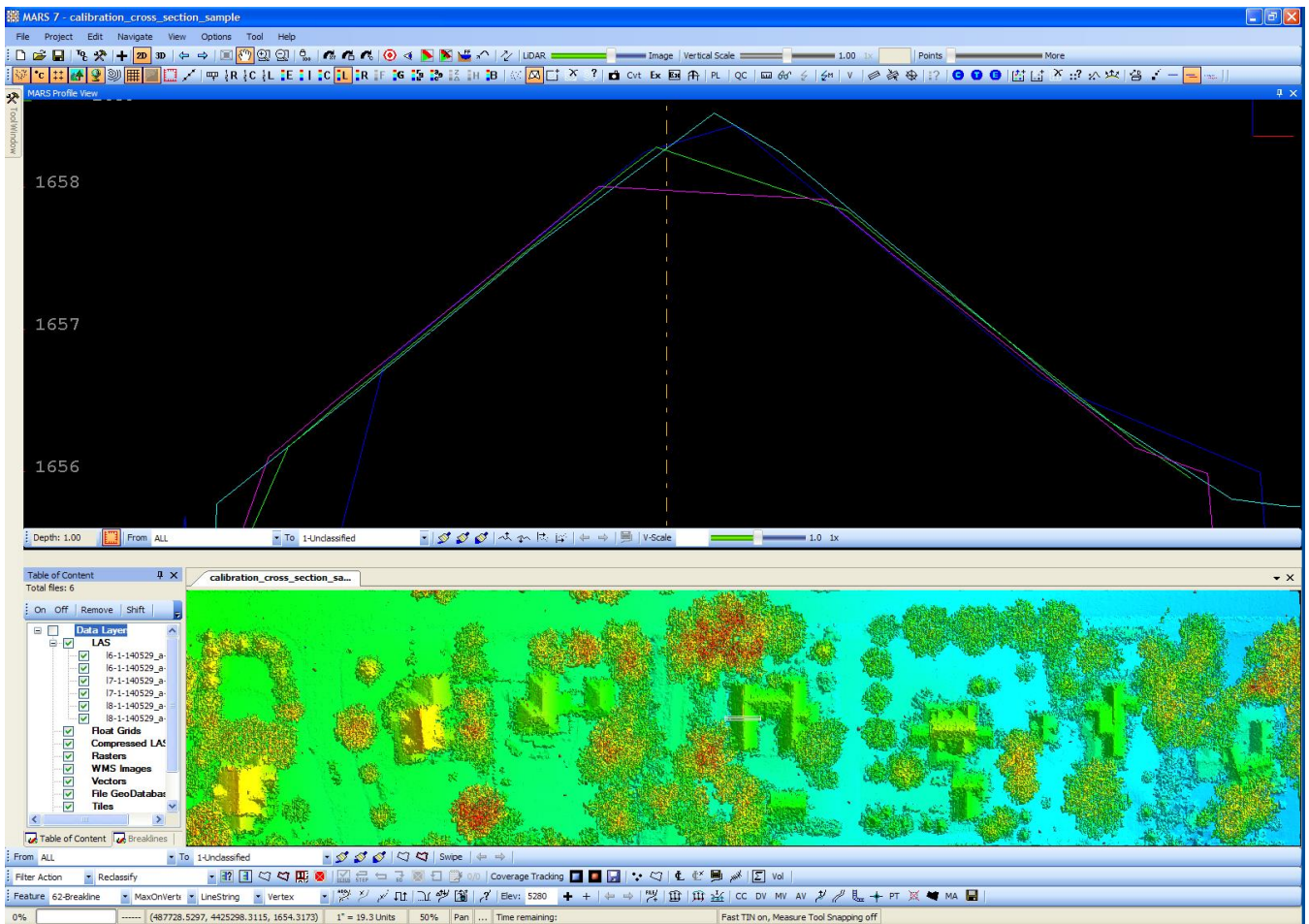
Roll and scale are checked on roof planes perpendicular to the flight direction.
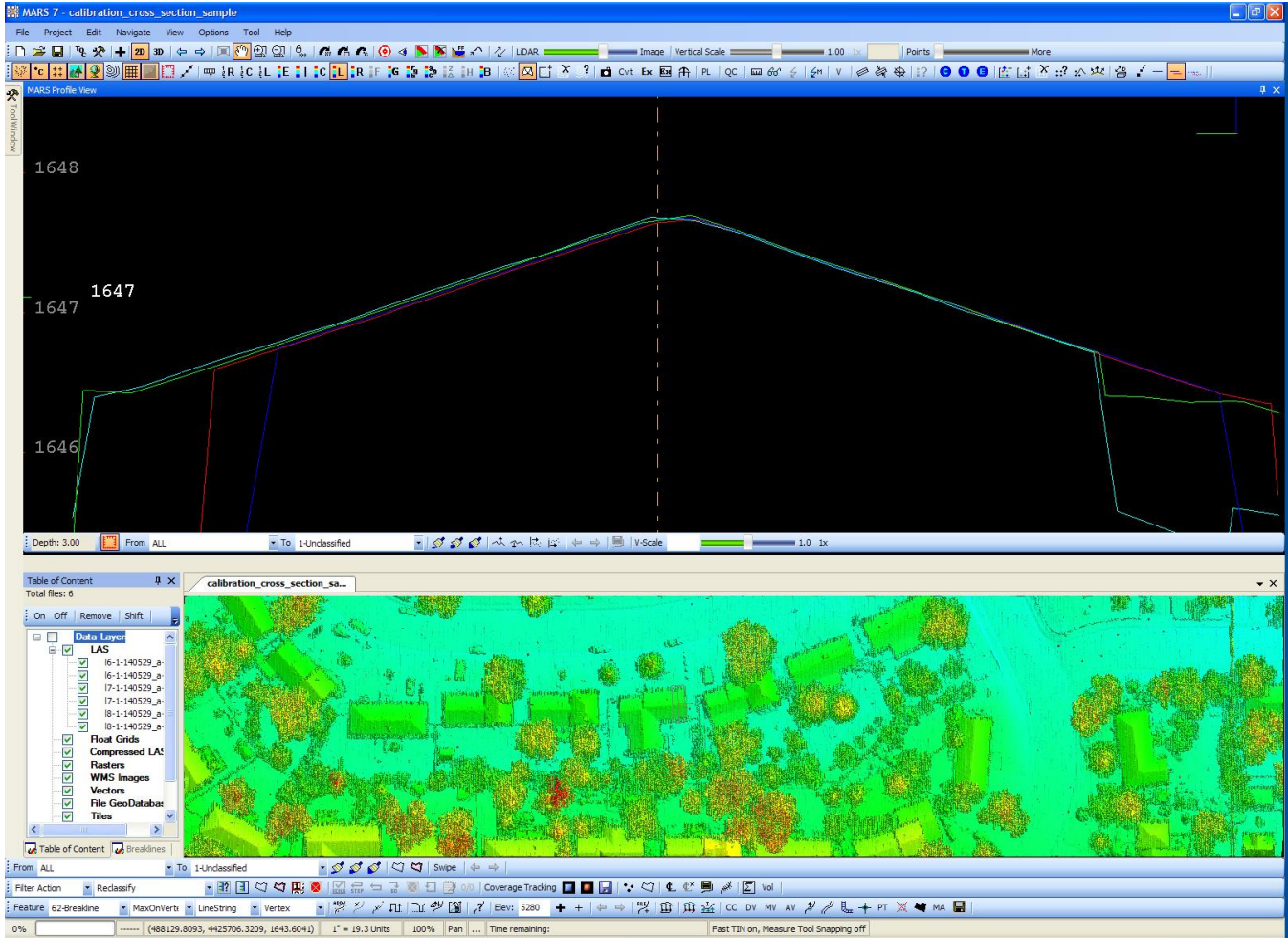
The LMS program outputs a "LCP" file with all the correction parameters. The calibration process may be run several times until the boresight adjustments are acceptable. When the boresight solution is acceptable the LCP file adjustments are saved and also applied to subsequent projects. Each new project is again analyzed and when the adjustment biases show too much drift a new boresight calibration is run. The LCP file may hold calibration tolerances for several projects.

Block LAS Production Processing Procedures

The LMS production mode is run on each flight line to further tie the final lidar LAS flight line files tightly together. Production settings allow scan angle scale, scan angle lag to float and allows elevation to move slightly during flight line to flight line comparison thus further tying flight lines together. A cross flight with locked elevation data is used for controlling flight line elevations.

A block of data is selected to process with LMS production settings. Data collected during turns at the ends of flight lines is deselected (light blue lines).
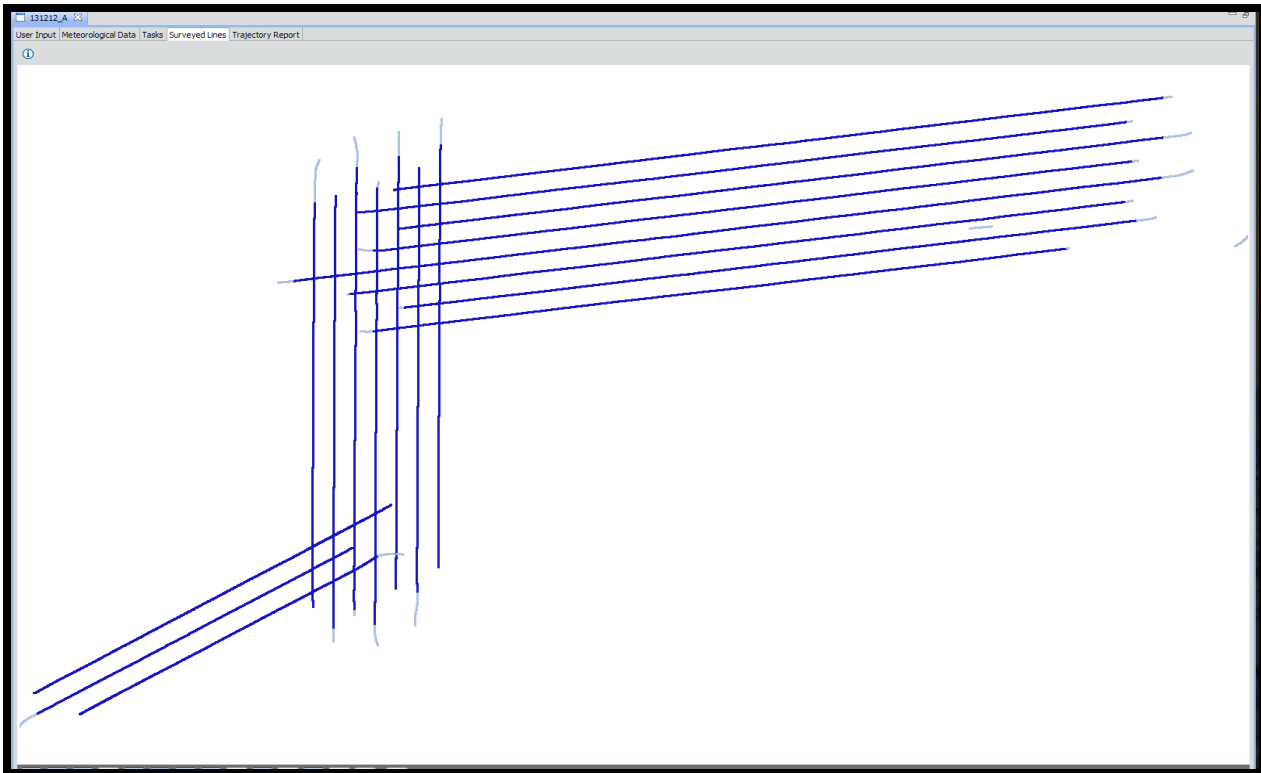
As in self-calibration the LMS production program analyses ground, roof planes and rooflines. One cross flight is locked in elevation and all other lines are adjusted to it. Unlike the calibration site the distribution of roof planes is usually much less dense. Here matched ground tie planes are blue.
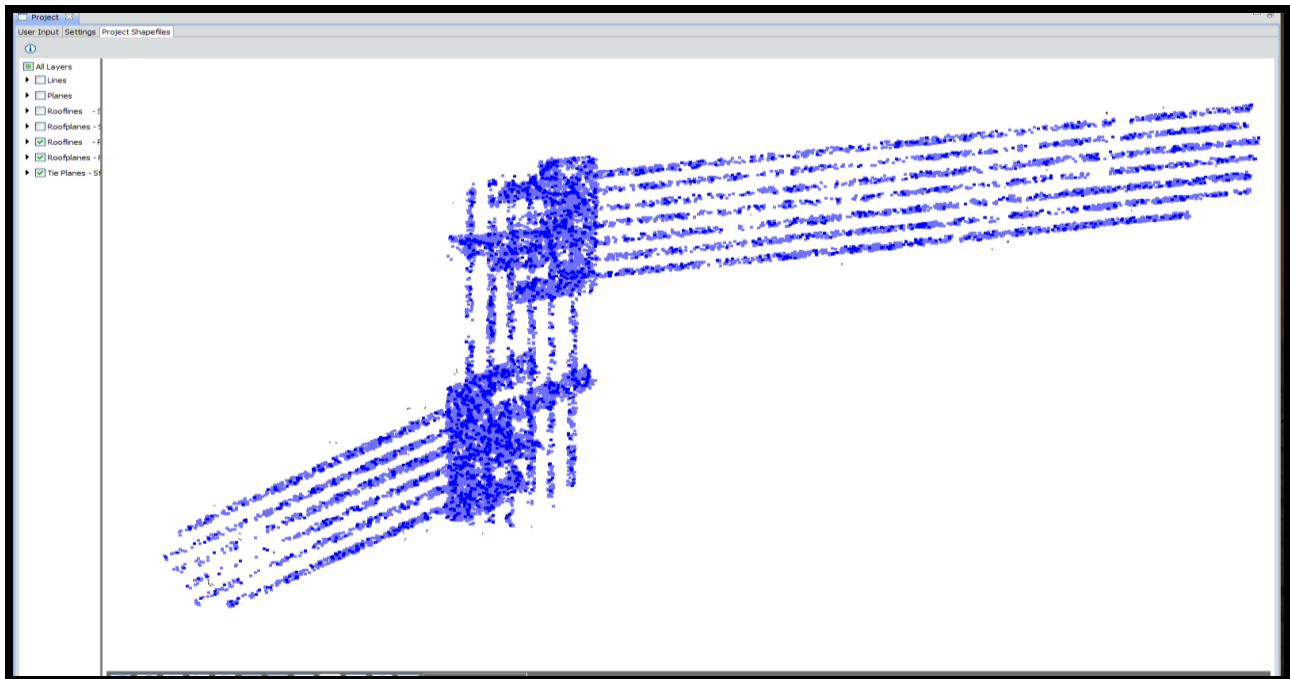

The same quality control outputs used to check self-calibrations are available to analyze the production run. Output plots are again available in LMS and cross sections plus a Flight Line Separation Raster are generated in MARS® to check coverage and quality.
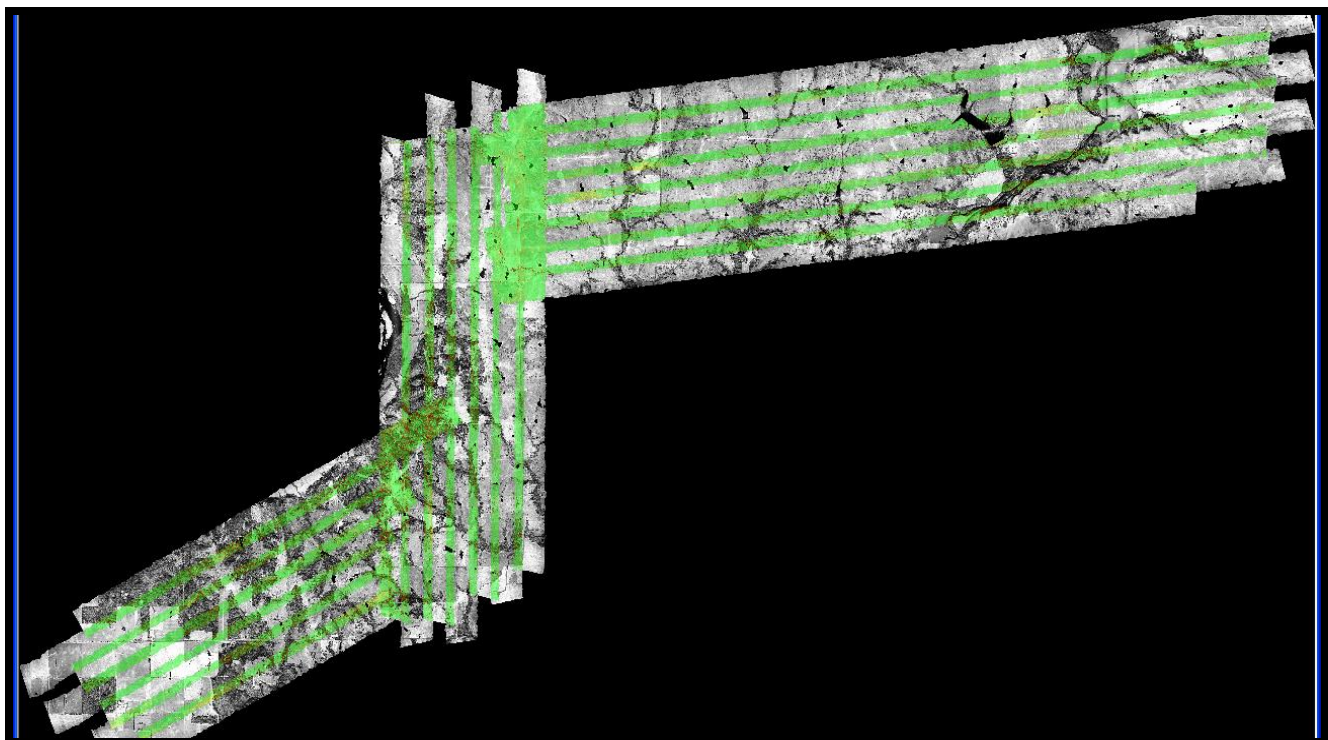
Correcting the Final Elevation

After all the lines are tied together a ground control network is imported into MARS®. The ground control network may be pre-existing or collected by a licensed surveyor.
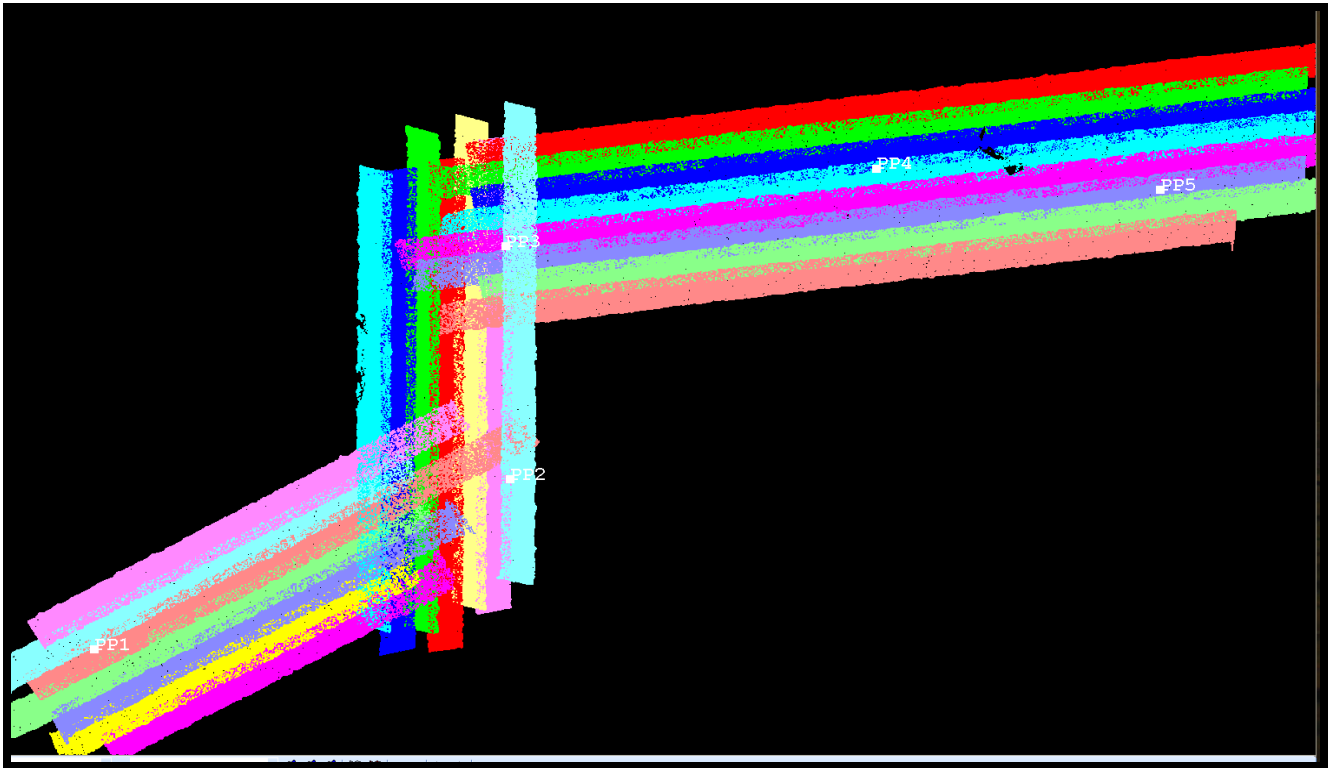
The next step is to match the ground control elevations to the lidar data set. A control report is run and the data set is shifted slightly to zero out the average elevation error and points checked for quality.

The final step before boresighted, leveled LAS files are ready for filtering is to run the MARS® QC Module on the block data. The Boresighted lidar QC Report outputs individual reports on Point Density, Nominal Pulse Spacing, Data Voids, Spatial Distribution, Scan Angles, Control Report, Flight Line Separation, Flight Line Overlap, Buffered Boundary, LAS Formats, Datums and Coordinates.
These reports are checked with the required specifications in the Project Management Plan.