

## Ground Control Point Survey Report

# “SCHOHARIE COUNTY NY QL2 LiDAR & SCHOHARIE CREEK WATERSHED EXPANSION QL2 LiDAR”

USGS Contract: G10PC00013

Task Order Number: G13PD00848 and G14PD00015

Prepared for:

*Unites States Geological Survey*



Prepared By:

**Dewberry Consultants LLC**

10003 Derekwood Lane, Suite 204

Lanham, Maryland, 20706

Phone (301)364-1855 Fax (301)731-0188

**TABLE OF CONTENTS**

1. Introduction

    1.1 Project Summary..... 3

    1.2 Points of Contact(s)..... 3

    1.3 Project Area .....4

2. Project Details

    2.1 Survey Equipment.....5

    2.2 Survey Point Details.....5

    2.3 Network Design.....5

    2.4 Field Survey Procedures and Analysis.....5

    2.5 Adjustment.....6

    2.6 Data processing Procedures.....6

3. Final Coordinates.....7

4. GPS Observation & Re-Observation Schedule.....7

5. Point Comparison Report.....7

6. Deliverables.....Sent via Electronic Transfer

    Including: a) Point Documentation Report & Photos of Survey Points

            b) Final Coordinate List in Excel Format

            c) NGS Data Sheets for Project Controls

# 1. INTRODUCTION

---

## 1.1 *Project Summary*

Dewberry Consultants, LLC is under contract to the United States Geological Survey to provide 5 Ground Control Points for USGS in the State of New York. Under the above referenced USACE Task Order, Dewberry is tasked to complete the quality assurance of Aerial Photography & Digital Orthophotography products. As part of this work Dewberry staff will complete Ground Control Point surveys that will be used to evaluate horizontal accuracy.

Existing NGS Control Points were located and surveyed to check the accuracy of the RTK/GPS survey equipment with the results shown in Section 2.4 of this Report.

As an internal QA/QC procedure and to verify that the Ground Control Points meet the 95% confidence level approximately 50% of the points were re-observed and are shown in Section 5 of this report.

Final horizontal coordinates are referenced to UTM Zone 18, NAD83 in meters. Final Vertical elevations are referenced to NAVD88 in meters using Geoid model 2012A (Geoid12A).

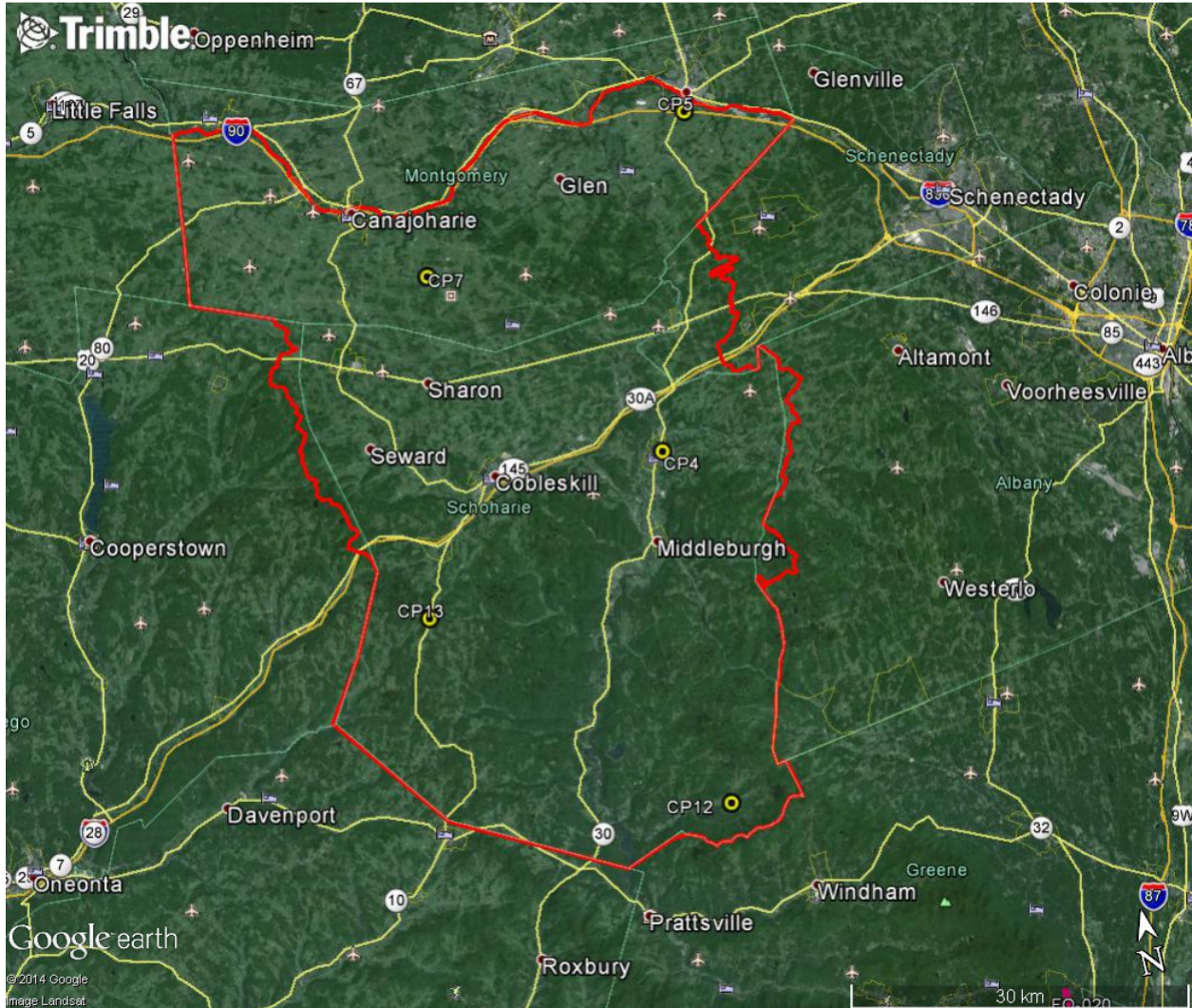
## 1.2 *Points of Contact*

Questions regarding the technical aspects of this report should be addressed to:

### **Dewberry Consultants LLC**

Gary Simpson, L.S.  
Senior Associate  
10003 Derekwood Lane  
Suite 204  
Lanham, Maryland 20706  
(301) 364-1855 direct  
(301) 731-0188 fax

### 1.3 Project Area



*USGS – Schoharie County and Schoharie Watershed*

## PROJECT DETAILS

---

### 2.1 *Survey Equipment*

In performing the GPS observations Trimble R-8 GNSS receiver/antenna attached to a two meter fixed height pole with a Trimble TSC3 Data Collector to collect GPS raw data were used to perform the field surveys.

### 2.2 *Survey Point Detail*

The 5 Ground Control Points were well distributed throughout the project area.

A sketch was made for each location and a nail was set at the point where possible or at an identifiable point. The Ground Control Point locations are detailed on the “Ground Control Point Documentation Report” sheets attached to this report.

### 2.3 *Network Design*

The GPS survey performed by Dewberry Consultants LLC office located in Lanham, MD was tied to a Real Time Network (RTN) managed by KeyNet GPS, Inc. The network is a series of “real-time” continuously operating, high precision GPS reference stations. All of the reference stations have been linked together using Trimble GPSNet software, creating a Virtual Reference Station System (VRS).

The Trimble NetR5 Reference Station is a multi-channel, multi-frequency GNSS (Global Navigation Satellite System) receiver designed for use as a stand-alone reference station or as part of a GNSS infrastructure solution. Trimble R-Track technology in the NetR5 receiver supports the modernized GPS L2C and L5 signals as well as GLONASS L1/L2 signals.

### 2.4 *Field Survey Procedures and Analysis*

Dewberry field surveyors used Trimble R-8 GNSS receivers, which is a geodetic quality dual frequency GPS receiver, to collect data at each surveyed location.

All locations were occupied once with approximately 50% of the locations being re-observed. All re-observations matched the initially derived station positions within the allowable tolerance of  $\pm 5$ cm or within the 95% confidence level. Each occupation which utilized the VRS network was occupied for approximately three (3) minutes in duration and measured to 180 epochs.

Each occupation which utilized OPUS (if used) was occupied between 18 and 20 minutes.

Field GPS observations are detailed on the “Ground Control Point Documentation Reports” submitted as part of this report.

Three (3) existing NGS monument listed in the NSRS database were located as an additional QA/QC method to check the accuracy of the VRS network as well as being the primary project control monuments designated as PID AA7904, AA7916 and, AA7916. The results are as follows:

NGS PT. ID	As Surveyed (M)			Published (M)			Differences (M)		
	Northing(M)	Easting(M)	Elev.(M)	Northing(M)	Easting(M)	Elev. (M)	Δ N	Δ E	Δ Elev.
COBLESKILL	4724027.695	542002.775	291.846	4,724,027.695	542,002.782	291.812	<b>0.000</b>	<b>0.007</b>	<b>0.034</b>
GPS2G93020	4755848.427	550539.105	89.316	4,755,848.443	550,539.106	89.351	<b>0.016</b>	<b>0.001</b>	<b>0.035</b>
WRIGHT	4728583.464	567197.903	409.879	4,728,583.476	567,197.915	409.70	<b>0.012</b>	<b>0.012</b>	<b>N/A</b>

The above results indicate that the VRS network is providing positional values within the 5cm parameters for this survey.

## 2.5 Adjustment

The survey data was collected using Virtual Reference Stations (VRS) methodology within a Virtual Reference System (VRS).

The system is designed to provide a true Network RTK performance, the RTKNet software enables high-accuracy positioning in real time across a geographic region. The RTKNet software package uses real-time data streams from the GPSNet system user and generates correction models for high-accuracy RTK GPS corrections throughout the network. Therefore, corrections were applied to the points as they were being collected, thus negating the need for a post process adjustment.

## 2.6 Data Processing Procedures

After field data is collected the information is downloaded from the data collectors into the office software. The Software program used is called TBC or Trimble Business Center.

Downloaded data is run through the TBC program to obtain the following reports; points report, point comparison report and a point detail report. The reports are reviewed for point accuracy and precision.

After review of the point data an “ASCII” or “txt” file which is the industry standard is created. Point files are loaded into our CADD program (Carlson Survey 2010) to make a visual check of the point data (Pt. #, Coordinates, Elev. and Description). The data can now be imported into the final product.

### 3. FINAL COORDINATES

POINT #	NORTHING (M)	EASTING (M)	ELEV. (M)	DESC.
<b>CONTROL POINTS (CP'S)</b>				
CP-4	4724126.160	557064.207	192.032	
CP-5	4752995.288	565327.777	125.566	
CP-7	4743607.921	540339.090	222.035	
CP-12	4692935.252	556077.450	489.582	
CP-13	4714448.650	533905.422	636.187	

### 4. GPS OBSERVATIONS

POINT ID	OBSERV. DATE	JULIAN DATE	TIME OF DAY	RE-OBSERV. DATE	RE-OBSERV. TIME
<b>CONTROL POINTS</b>					
CP-4	5/18/2014	138	10:50	5/22/2014	5:20
CP-5	5/19/2014	139	11:01	5/22/2014	6:01
CP-7	5/18/2014	138	10:58	5/22/2014	6:59
CP-12	5/20/2014	140	16:36	5/22/2014	8:33
CP-13	5/22/2014	142	14:44	5/22/2014	7:44

### 5. POINT COMPARISON

POINT ID	POINT CK	DELTA NORTH (M)	DELTA EAST (M)	VERT. DIFF (M)
CP-4	CP-4CK	0.002	0.005	0.002
CP-5	CP-5CK	0.005	0.003	0.020
CP-7	CP-7CK	0.008	0.001	0.002
CP-12	CP-12CK	0.024	0.006	0.039
CP-13	CP-13CK	0.002	0.004	0.014