

Lewis and Clark, MT 2012 Imagery Project

Aerial Triangulation Report

July 23, 2012

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Sanborn has successfully completed the aerial triangulation (AT) task for the aerial photography acquired May 7th through 11th, 2012 for the Lewis and Clark project.

Using fully analytical aerial triangulation (FAAT) methods incorporating automatic analytical aerial triangulation (AAAT) procedures, Sanborn determined ground coordinates for each exposure by flying at an average altitude of 3100m AMSL covering the project area.

The results of the final adjustment are sufficient to enable Sanborn to produce digital photogrammetric mapping with an appropriate ground pixel resolution that meets project accuracy requirements (ASPRS Class 1 for 1:1200 mapping).

AT Accuracy Statement

The mean standard deviation of all adjusted terrain points indicate the AT solution exceeds the accuracy requirements for ASPRS Class 1 1:1200 scale mapping.

1. FLIGHT/CONTROL MAP

Dates of photography: May 7th through 11th, 2012 Number of flight lines: Helena: 53, Limestone: 20 Number of exposures: Helena: 2250, Limestone: 552 Flight Height: 3100m AMSL Image Overlap: 60% FOL / 30% SOL



2. AIRBORNE – GPS/INS PROCESSING

The airborne-GPS data were processed using POSPac[™] (version 5.4) Mobile Mapping Suite; GPS-IMU tightly coupled processing software which uses Kalman Filtering techniques, and On-The-Fly (OTF) ambiguity resolution techniques. Multiple CORS stations are being used in SmartBase trajectory processing.

2.1 SmartBase Processing Technique

Applanix SmartBase processing mode creates a virtual base station, which follows plane trajectory allowing faster and more accurate on the flight kinematic ambiguity resolution. In order to process trajectory in SmartBase processing mode multiple CORS stations (usually from 6 to 11 CORS stations per mission) are imported into the project. The network of the CORS stations creates a closed polygon around the plane trajectory. Within the polygon atmospheric corrections are well modeled and applied to each photo center. One of the most reliable CORS stations is chosen as primary station. The SmartBase quality check wrt primary is performed on all CORS stations involved in the network. Any CORS stations failing QAQC check are eliminated from processing. In the following step Applanix SmartBase CORS network adjustment is run to adjust all CORS stations to a common datum. The final step in Applanix SmartBase processing is 'GNSS-Inertial Processor' which combines GPS CORS data with inertial data in tightly coupled process. SmartBase processing creates a virtual base station, which follows plane trajectory within SmartBase region polygon. All CORS stations contribute to virtual base station accuracy.

The precise position of the camera lens node was interpolated from the trajectory of GPS positions utilizing polynomial fitting techniques. The time-tag for each event served as a basis for the interpolation.

The lever arm offset values are applied to this data resulting in a final AGPS file containing the coordinates of the camera lens node at each instant of exposure. Final Exterior Orientation parameters and positions are outputted using project assigned datum, projections and units.

2.2 ABGPS Ground Reference Stations

POSPac 5.4 SmartBase processing technique requires multiple GPS ground reference stations of at least 18 hours of data to process plane trajectory. The networks of CORS stations imported into the project, created 'SmartBase Region' polygon for each mission. The 'SmartBase Region' assists in virtual base station creation and atmospheric corrections model.

3. AERIAL TRIANGULATION

3.1 Overview

Aerial triangulation is the simultaneous space resection of image rays projected and recorded at one source, the perspective center of the aerial camera. These image rays projected from two or more overlapping images, stereo-models, intersect at the corresponding ground location to determine the three-dimensional coordinates of each point measured. This collection of image rays is fit to known ground survey control in a simultaneous 3-dimensional least squares adjustment. After the completion of this adjustment, coordinates of the 'unknown' ground points are derived by the intersection of the adjusted image points.

The purpose of aerial triangulation is to densify horizontal and vertical control from relatively few ground control points (GCPs). Since obtaining GCPs is a relatively significant expense in any mapping project, AT procedures are used to reduce the amount of field survey required by extending control to all stereo-models.

This method is essentially a mathematical tool, capable of extending control to areas between ground survey points using several contiguous uncontrolled stereo-models.

3.2 Simultaneous Adjustment by Bundles

The surveyed control, along with the reduced image coordinates, served as input into the 'combined' block adjustment. Three–dimensional, simultaneous least squares adjustments by bundles, commonly referred to as "bundle" adjustments, were undertaken using Match-AT adjustment software. This particular bundle method is very sensitive to systematic errors of the photo measurements and provides the correction of constant and regular errors through self-calibration. This concept regards these types of errors to be common to all photographs or to be present in sub-sets of the photographs. This bundle block adjustment software has proven to be a very rigorous and stable platform.

A series of aerial triangulation solutions were completed. The adjustment strategy was devised to provide the optimal solution for the subsequent mapping, while providing comprehensive quality control to detect errors, omissions and spurious data.

3.2.1 Fully Constrained Adjustment

The final adjustment, and the optimal solution to be used for mapping, included all control points as constraints.

All Ground Control Points were assigned a Weighting Factor of 0.10m in X, Y, and Z.

All image points were assigned standard deviations of $4\mu m$.

Helena Block

residuals horizontal control points in [meter]

| control point ID | rx | ry |
|------------------|--------|--------|
| H01 | -0.050 | -0.187 |
| H02 | 0.069 | 0.068 |
| H03 | -0.007 | -0.229 |
| H04 | -0.082 | -0.039 |
| H05 | -0.067 | -0.005 |
| H06 | -0.070 | -0.068 |
| E100 | -0.238 | 0.046 |
| E113 | -0.090 | 0.016 |
| E120 | 0.023 | -0.004 |
| E114R | 0.127 | 0.111 |
| E116R | 0.121 | 0.101 |
| E123R | 0.264 | 0.191 |

residuals vertical control points in [meter]

| control point | ID | rz |
|--|----|---|
| H01 H02 H03 H04 H05 H06 E100 E113 E120 E114R E116R | | 0.031 -0.063 -0.110 0.028 -0.039 -0.035 0.016 0.144 0.085 0.023 0.012 |
| E123R | | -0.094 |

Limestone Block

residuals horizontal control points in [meter]

| control point ID | rx | ry |
|------------------|--------|--------|
| LT01 | -0.016 | 0.189 |
| LT02 | 0.063 | -0.182 |
| LT03 | -0.165 | -0.151 |
| LT04 | -0.132 | -0.003 |
| LT05 | 0.074 | 0.136 |
| LT06 | 0.175 | 0.011 |

residuals vertical control points in [meter]

| control point ID | rz |
|------------------|--------|
| LT01 | 0.036 |
| LT02 | -0.005 |
| LT03 | 0.044 |
| LT04 | 0.019 |
| LT05 | 0.002 |
| LT06 | -0.095 |

3.2.2 Project Ground Control

Helena Block

| Point | Easting | Northing | Elevation |
|-------|------------|------------|-----------|
| E100 | 408629.414 | 256699.438 | 1410.968 |
| E113 | 399201.797 | 267877.958 | 1255.814 |
| E114R | 420019.182 | 265754.345 | 1201.870 |
| E116R | 410540.966 | 267167.375 | 1139.469 |
| E120 | 402544.727 | 277388.370 | 1221.635 |
| E123R | 410387.306 | 280263.226 | 1241.793 |
| H01 | 399512.416 | 285834.965 | 1415.547 |
| H02 | 419601.478 | 279374.378 | 1152.787 |
| Н0З | 391161.680 | 275454.501 | 1619.363 |
| H04 | 391812.319 | 261862.002 | 1357.931 |
| H05 | 399179.508 | 253317.512 | 1564.673 |
| H06 | 419251.956 | 252627.866 | 1583.512 |
| | | | |

Limestone Block

| Point | Easting | Northing | Elevation |
|--------------------------------------|--|--|--|
| LT01 LT02 LT03 LT04 LT05 | 443186.057 433455.713 438195.977 444849.509 446877.573 | 233976.038 224088.551 219926.338 221402.841 228615.398 | 1183.936 1424.844 1308.445 1191.875 1179.600 |
| LT06 | 438451.785 | 233858.119 | 1355.783 |

3.3 Final Coordinates and Elevations

Montana State Plane Coordinates, NAD 83 HARN, meters

3.4 Summary of AT Results

Helena Block

| Active Block | : Block_All |
|---|---|
| Number of photos Number of strips | : 2271 : 54 |
| Photo scale Mean terrain height [m] | : 1:15716 : 1300 |
| Automatic blunder detection | : OFF |
| Use all adjusted points in project file as control (absolute mode) | : OFF |
| Control parameter for block adjustment : | |
| Selfcalibration GNSS-Mode Drift-Mode drift per block | : OFF : ON : ON : ON only shifts |
| are enabled drift for X,Y,Z IMU-Mode IMU-Boresight Earth's curvature correction Atmospheric correction | : ON,ON,ON : ON : ON |

Standard deviations (a-priori) :

| Ground control (planimetry) [m] | | |
|---------------------------------|---|-------|
| Set 0 (=default) | : | 0.100 |
| Ground control (height) [m] | | |
| Set 0 (=default) | : | 0.100 |

Automatic image points [mm] Set 0 (=default) : 0.004 Image points of ground control and manual measurements [mm] : 0.004 GNSS X Y Z [m] : 0.300 0.300 0.300 INS omega phi kappa [deg] : 0.008 0.008 0.008 Used Cameras in block: _____ 1 UCD-SU-1-0022 Distortion : No correction

total of 236496 measurements in 2271 photos are used for adjustment (total 2271 photos)

| sigma | naught | 1.8 | micron | (10:58:51) |
|-------|--------|-----|--------|------------|
| sigma | naught | 1.7 | micron | (10:58:57) |

| found | 12559 | points | connecting | 2 | photos |
|-------|-------|--------|------------|----|--------|
| found | 17185 | points | connecting | 3 | photos |
| found | 11089 | points | connecting | 4 | photos |
| found | 15193 | points | connecting | 5 | photos |
| found | 5805 | points | connecting | 6 | photos |
| found | 238 | points | connecting | 7 | photos |
| found | 184 | points | connecting | 8 | photos |
| found | 86 | points | connecting | 9 | photos |
| found | 26 | points | connecting | 10 | photos |
| found | 7 | points | connecting | 11 | photos |
| found | 1 | points | connecting | 12 | photos |
| | | | | | |

| number | of | observations | 485832 |
|---------|------|--------------|--------|
| number | of | unknowns | 200754 |
| redunda | ancy | 7 | 285078 |

RMS automatic points in photo (number: 229760)

Х 1.2 micron 1.2 micron У RMS control and manual points in photo (number: 6325) Х 2.9 micron 2.6 micron У RMS control points with default standard deviation set (number: 12) 0.126 [meter] Х У 0.115 [meter] RMS control points with default standard deviation set (number: 12) Z 0.070 [meter] RMS IMU observations (number: 2271) omega 0.005 [deg] phi 0.004 [deg] 0.007 [deg] kappa RMS GNSS observations (number: 2271) Х 0.144 [meter] 0.151 [meter] У 0.100 [meter] Z

Limestone Block

| Active Block Block | : | complete |
|---|---|---|
| Number of photos Number of strips | | 552 20 |
| Photo scale Mean terrain height [m] | | 1:17085 1200 |
| Automatic blunder detection | : | OFF |
| Use all adjusted points in project file as control (absolute mode) | : | OFF |
| Control parameter for block adjustment : | | |
| Selfcalibration GNSS-Mode Drift-Mode drift per block are enabled drift for X,Y,Z | : | OFF ON ON ON only shifts ON,ON,ON |

| IMU-Mode | : | ON |
|--------------------------------|---|-----|
| IMU-Boresight | : | ON |
| Earth's curvature correction | : | ON |
| Atmospheric correction | : | ON |
| Do not eliminate manual points | : | OFF |

Standard deviations (a-priori) :

_____ Ground control (planimetry) [m] Set 0 (=default) : 0.100 Ground control (height) [m] Set 0 (=default) : 0.100 Automatic image points [mm] Set 0 (=default) : 0.004 Image points of ground control and manual measurements [mm] : 0.004 X Y Z [m] : 0.300 0.300 GNSS 0.300 INS omega phi kappa [deg] : 0.008 0.008 0.008 Used Cameras in block: _____ 1 UCD-SU-1-0022 Distortion : No correction

total of 61446 measurements in 552 photos are used for adjustment (total 552 photos)

sigma naught 1.6 micron (09:53:08) sigma naught 1.4 micron (09:53:09) found 2514 points connecting 2 photos 3937 points connecting 3 photos found found 3395 points connecting 4 photos found 3913 points connecting 5 photos 1543 points connecting 6 photos 202 points connecting 7 photos 74 points connecting 8 photos found found found found 22 points connecting 9 photos number of observations 126222 number of unknowns 50121 76101 redundancy RMS automatic points in photo (number: 60361) 1.1 micron Х 1.1 micron У RMS control and manual points in photo (number: 1085) 1.0 micron х 1.3 micron У RMS control points with default standard deviation set (number: 6) Х 0.119 [meter] V 0.136 [meter] RMS control points with default standard deviation set (number: 6) 0.046 [meter] Z RMS IMU observations (number: 552) omega 0.004 [deg] 0.005 [deg] phi 0.006 [deg] kappa RMS GNSS observations (number: 552) x 0.144 [meter] 0.170 [meter] У Z 0.121 [meter]

UltraCam D, Serial Number UCD-SU-1-0022



Calibration Report

Short version



Camera:

UltraCam D, S/N UCD-SU-1-0022

Manufacturer:

Date of Calibration: Date of Report: Camera Revision: Revision of Report: Vexcel Imaging GmbH, A-8010 Graz, Austria

Sep-30-2008 Nov-25-2008 5.0 5.0





Panchromatic Camera

Large Format Panchromatic Output Image

| Image Format | long track | 67.5mm | 7500 pixel |
|-----------------|--|--------------------|------------------|
| | cross track | 103.5mm | 11500 pixel |
| | | | |
| Image Extent | | (-33.75, -51.75)mm | (33.75, 51.75)mm |
| | | | |
| Pixel Size | | 9.000µm*9.000µm | |
| | | | |
| Focal Length | ck | 101.400mm | ± 0.002mm |
| | | | |
| Principal Point | X_ppa | 0.000 mm | ± 0.002mm |
| (Level 2) | Y_ppa | 0.180 mm | ± 0.002mm |
| | | | |
| Lens Distortion | n Remaining Distortion less than 0.002mm | | |

Multispectral Camera

Medium Format Multispectral Output Image (Upscaled to panchromatic image format)

| Image Format | long track | 67.5mm | 2400 pixel |
|-----------------|--|--------------------|------------------|
| | cross track | 103.5mm | 3680 pixel |
| | | | |
| Image Extent | | (-33.75, -51.75)mm | (33.75, 51.75)mm |
| | | | |
| Pixel Size | | 28.125µm*28.125µm | |
| | | | |
| Focal Length | ck | 101.400mm | |
| | | | |
| Principal Point | X_ppa | 0.000 mm | ± 0.002mm |
| (Level 2) | Y_ppa | 0.180 mm | ± 0.002mm |
| | | | |
| Lens Distortion | n Remaining Distortion less than 0.002mm | | |

UltraCam D, Serial Number UCD-SU-1-0022



Explanations:

1) Calibration Method:

The geometric calibration is based on a set of 84 images of a defined geometry target with 240 GCPs.

| Number of point measurements for the panchromatic camera : | 19415 |
|---|-------|
| Number of point measurements for the multispectral camera : | 64864 |

Determination of the image parameters by Least Squares Adjustment. Software used for the adjustment: BINGO (GIP Eng. Aalen, Germany)

| 2) Level 2 Image Coordinate System: | pan MS | 11500 pixel by 3680 pixel by | |
|-------------------------------------|-----------|---------------------------------|------------|
| | | occo pixer by | LHOU PINCI |

LvI2, Camera prop. Orientation



The image coordinate system of the Level 2 images is shown in the above figure. The level 2 image consists of 11500 columns and 7500 rows, which leads to a total image format of 103.5 * 67.5 mm. The coordinate of the principal point in the level 2 image is given on page 3 of this report. The above figure shows the position of an example principal point at the coordinate (-0.123 / 0.345). UltraCam D, Serial Number UCD-SU-1-0022



Calibration Report

Summary



Camera:

UltraCam D, S/N UCD-SU-1-0022

Manufacturer: Vexcel Imaging GmbH, A-8010 Graz, Austria

| Date of Calibration: | Sep-30-2008 |
|-------------------------------------|--------------------|
| Date of Report: Camera Revision: | Nov-25-2008 5.0 |
| Revision of Report: | 5.0 |

The following calibrations have been performed for the above mentioned digital aerial mapping camera:

- Geometric Calibration
- Verification of Lens Quality and Sensor Adjustment
- Radiometric Calibration
- Calibration of Defective Pixel Elements
- Shutter Calibration
- Sensor and Electronics Calibration

This equipment is operating fully within specification as defined by Vexcel Imaging GmbH.

Dr. Michael Gruber

Chief Scientist, Photogrammetry Vexcel Imaging GmbH. DI (FH) Michael Kröpfl Senior Calibration Engineer Vexcel Imaging GmbH