

LiDAR Remote Sensing Data Collection
Department of Geology and Mineral Industries
Newberry Study Area
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LIDAR REMOTE SENSING DATA COLLECTION: DOGAMI, NEWBERRY STUDY AREA

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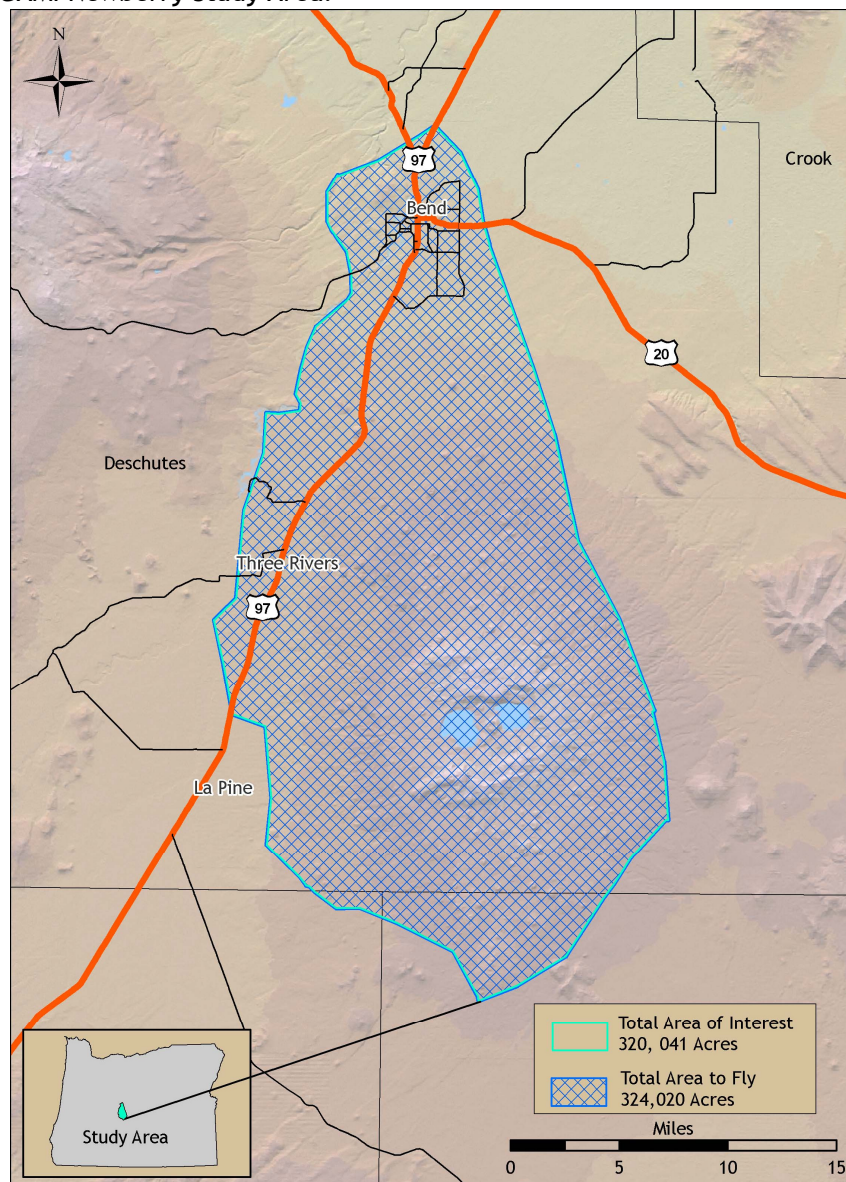
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1. Overview

1.1 Study Area

Watershed Sciences, Inc. has collected Light Detection and Ranging (LiDAR) data of the Newberry Study Area for the Oregon Department of Geology and Mineral Industries (DOGAMI). The area of interest (AOI) totals 500 square miles (320,041 acres) and the total area flown (TAF) covers 506 square miles (324,020 acres). The TAF acreage is greater than the original AOI acreage due to buffering and flight planning optimization (Figure 1.1 below). This report will be amended to reflect new data and cumulative statistics for the overall LiDAR survey with every delivery. DOGAMI data are delivered in OGIC (HARN): Projection: Oregon Statewide Lambert Conformal Conic; horizontal and vertical datum: NAD83 (HARN)/NAVD88 (Geoid03); units: International Feet.

Figure 1.1. DOGAMI Newberry Study Area.



1.2 Area Delivered to Date

Total delivered acreage to date is detailed below.

DOGAMI Newberry Study Area				
	Delivery Date	Acquisition Dates	AOI Acres	TAF Acres
Delivery Area 1	September 8, 2010	May 28, 2010 - June 1, 2010	31,380	32,225
Delivery Area 2	October 20, 2010	May 28, 2010 - July 27, 2010	159,215	160,744

Figure 1.2. Newberry Study Area, illustrating the delivered portions of the TAF.

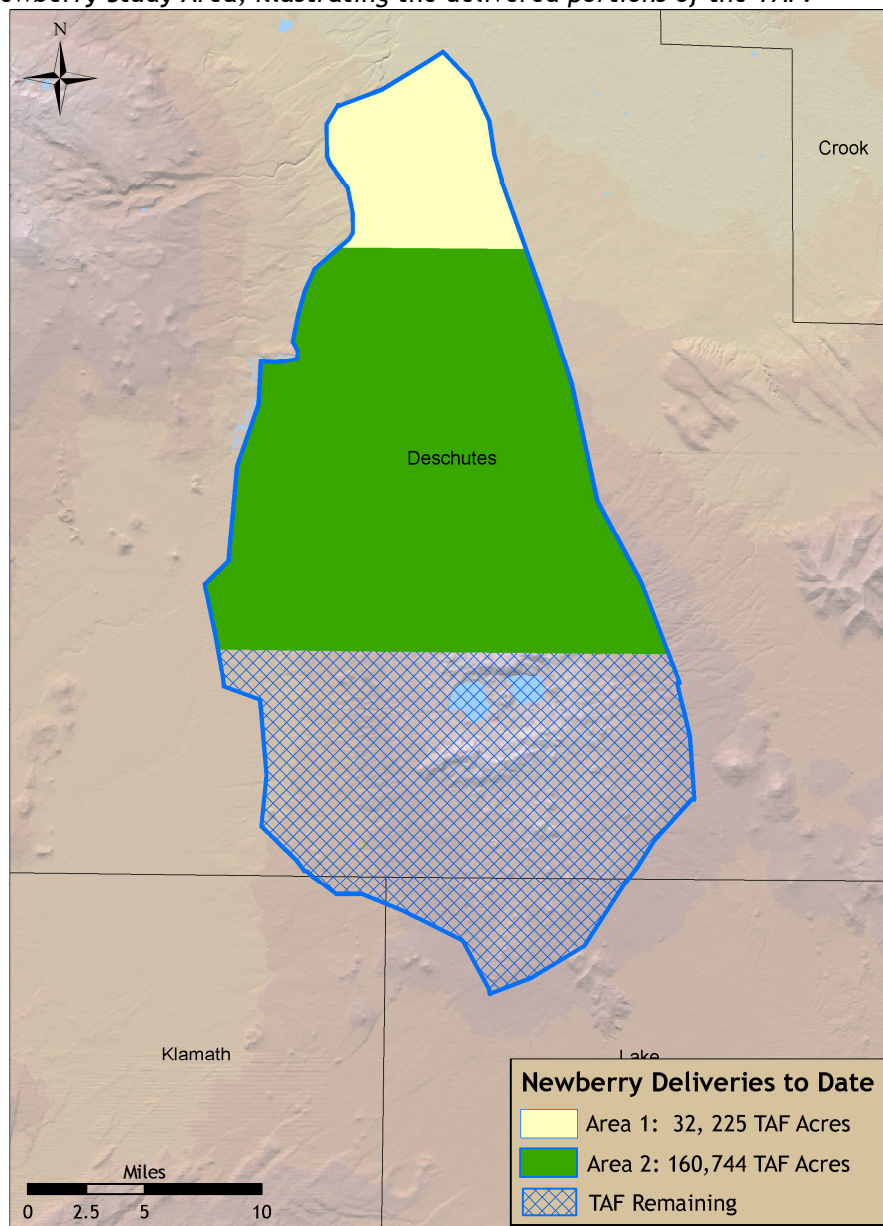
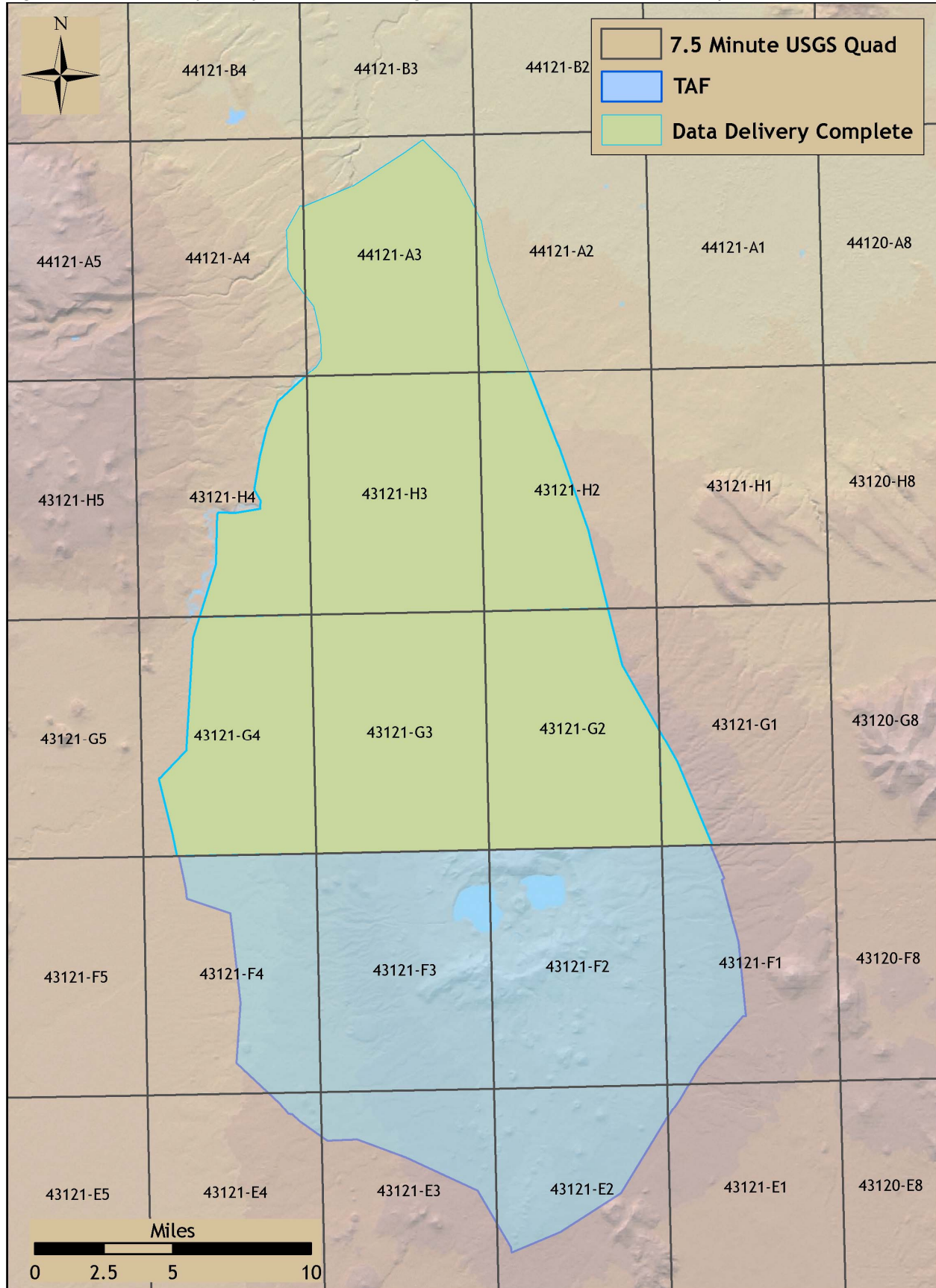


Figure 1.3. Newberry Study Area, illustrating the delivered 7.5 minute USGS quads.



2. Acquisition

2.1 Airborne Survey Overview - Instrumentation and Methods

The LiDAR survey utilized a Leica ALS60 and an ALS50 Phase II sensor mounted in Cessna Caravan 208B. The Leica systems were set to acquire $\geq 83,000$ laser pulses per second (i.e. 83 kHz pulse rate) and flown at 900 and 1300 meters above ground level (AGL), capturing a scan angle of $\pm 14^\circ$ from nadir¹. These settings are developed to yield points with an average native density of ≥ 8 points per square meter over terrestrial surfaces. The native pulse density is the number of pulses emitted by the LiDAR system. Some types of surfaces (i.e. dense vegetation or water) may return fewer pulses than the laser originally emitted. Therefore, the delivered density can be less than the native density and lightly variable according to distributions of terrain, land cover and water bodies.



The Cessna Caravan is a powerful, stable platform, which is ideal for the often remote and mountainous terrain found in the Pacific Northwest. The Leica ALS60 sensor head installed in the Caravan is shown on the right.

Table 2.1 LiDAR Survey Specifications

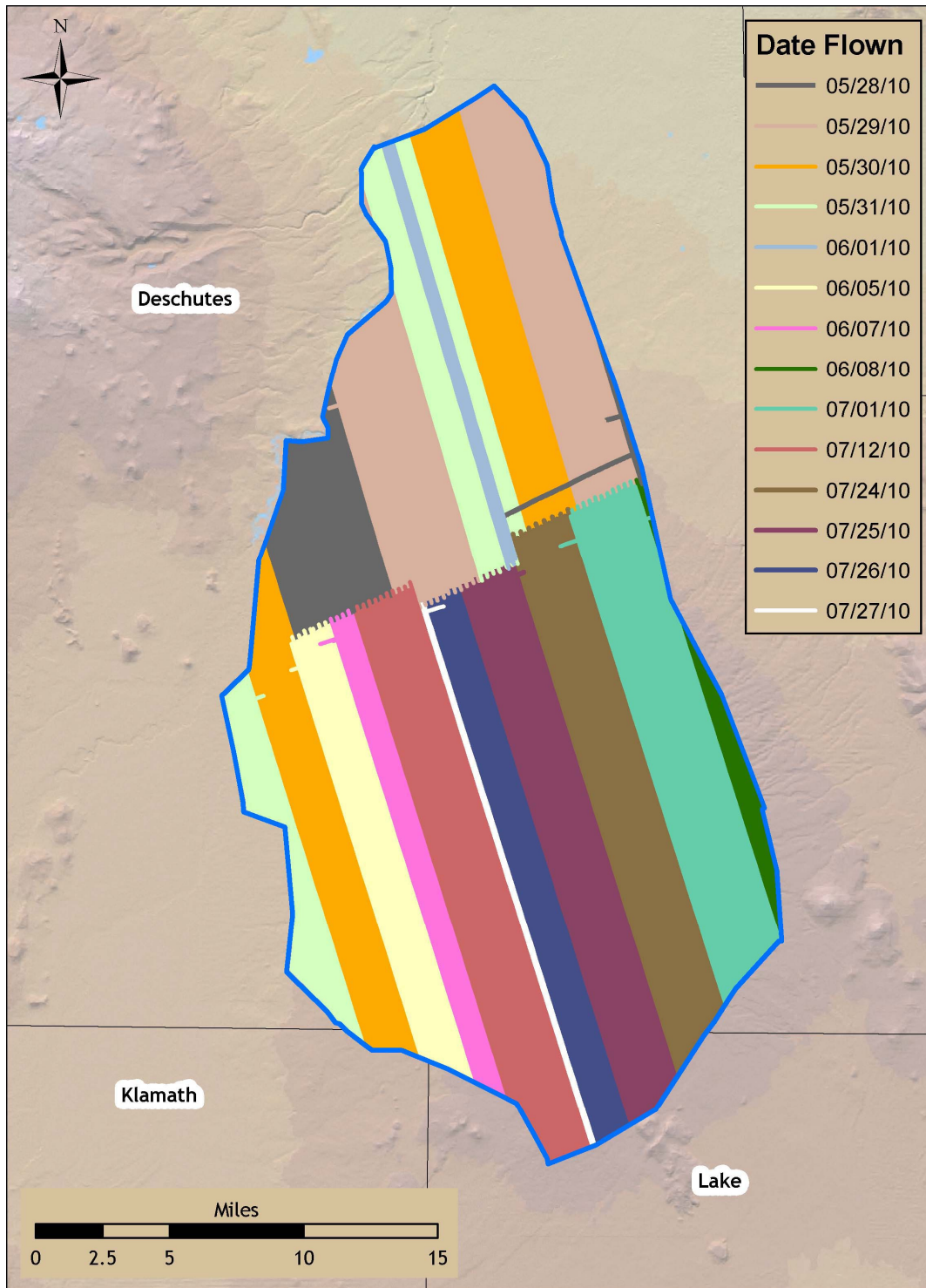
Sensors	Leica ALS60 and ALS50 Phase II
Survey Altitude (AGL)	900 m and 1300 m
Pulse Rate	>83 kHz
Pulse Mode	Single
Mirror Scan Rate	52 Hz
Field of View	$28^\circ (\pm 14^\circ \text{ from nadir})$
Roll Compensated	Up to 15°
Overlap	100% (50% Side-lap)

The study area was surveyed with opposing flight line side-lap of $\geq 50\%$ ($\geq 100\%$ overlap) to reduce laser shadowing and increase surface laser painting. The system allows up to four range measurements per pulse, and all discernable laser returns were processed for the output dataset.

To solve for laser point position, it is vital to have an accurate description of aircraft position and attitude. Aircraft position is described as x, y and z and measured twice per second (2 Hz) by an onboard differential GPS unit. Aircraft attitude is measured 200 times per second (200 Hz) as pitch, roll and yaw (heading) from an onboard inertial measurement unit (IMU). **Figure 2.1** shows the flight lines completed for current processing.

¹ Nadir refers to the perpendicular vector to the ground directly below the aircraft. Nadir is commonly used to measure the angle from the vector and is referred to a “degrees from nadir”.

Figure 2.1. Actual flightlines for the Newberry Study Area illustrating the study area.



2.2 Ground Survey - Instrumentation and Methods

During the LiDAR survey, static (1 Hz recording frequency) ground surveys were conducted over either known or set monuments. Monument coordinates are provided in **Table 2.2** and shown in **Figure 2.2** for the AOI. After the airborne survey, the static GPS data are processed using triangulation with continuous operation stations (CORS) and checked using the Online Positioning User Service (OPUS²) to quantify daily variance. Multiple sessions are processed over the same monument to confirm antenna height measurements and reported position accuracy. Control monuments are located within 13 nautical miles of the survey area(s). Indexed by time, these GPS data records are used to correct the continuous onboard measurements of aircraft position recorded throughout the mission.

2.2.1 Instrumentation

For this delivery area all Global Navigation Satellite System (GNSS³) survey work utilizes a Trimble GPS receiver model R7 with a Zephyr Geodetic antenna with ground plane for static control points. The Trimble GPS R8 unit is used primarily for Real Time Kinematic (RTK) work but can also be used as a static receiver. For RTK data, the collector begins recording after remaining stationary for 5 seconds then calculating the pseudo range position from at least three epochs with the relative error under 1.5 cm horizontal and 2 cm vertical. All GPS measurements are made with dual frequency L1-L2 receivers with carrier-phase correction.



² Online Positioning User Service (OPUS) is run by the National Geodetic Survey to process corrected monument positions.

³ GNSS: Global Navigation Satellite System consisting of the U.S. GPS constellation and Soviet GLONASS constellation

2.2.2 Monumentation

Whenever possible, existing and established survey benchmarks shall serve as control points during LiDAR acquisition including those previously set by Watershed Sciences. In addition to NGS, the county surveyor's offices and the Oregon Department of Transportation (ODOT) often establish their own benchmarks. NGS benchmarks are preferred for control points. In the absence of NGS benchmarks, county surveys, or ODOT monumentation, Watershed Sciences produces our own monuments. These monuments are spaced at a minimum of one mile and every effort is made to keep these monuments within the public right of way or on public lands. If monuments are required on private property, consent from the owner is required. All monumentation is done with 5/8" x 24" or 30" rebar topped with an orange or blue plastic cap stamped "WS" or with an aluminum cap stamped with the point name.



2.2.3 Methodology

Each aircraft is assigned a ground crew member with two R7 receivers and an R8 receiver. The ground crew vehicles are equipped with standard field survey supplies and equipment including safety materials. All data points are observed for a minimum of two survey sessions lasting no fewer than 6 hours. At the beginning of every session the tripod and antenna are reset, resulting in two independent instrument heights and data files. Data are collected at a rate of 1Hz using a 10 degree mask on the antenna.

The ground crew uploads the GPS data to the FTP site on a daily basis to be returned to the office for Professional Land Surveyor (PLS) oversight, Quality Assurance/Quality Control (QA/QC) review and processing. OPUS processing triangulates the monument position using 3 CORS stations resulting in a fully adjusted position. CORPSCON⁴ 6.0.1 software is used to convert the geodetic positions from the OPUS reports. After multiple days of data have been collected at each monument, accuracy and error ellipses are calculated. This information leads to a rating of the monument based on FGDC-STD-007.2-1998⁵ Part 2 table 2.1 at the 95% confidence level.

All GPS measurements are made during periods with PDOP less than or equal to 3.0 and with at least 6 satellites in view of both a stationary reference receiver and the roving receiver. RTK positions are collected on 20% of the flight lines and on bare earth locations such as paved, gravel or stable dirt roads, and other locations where the ground is clearly visible (and is likely to remain visible) from the sky during the data acquisition and RTK measurement period(s). In order to facilitate comparisons with LiDAR measurements, RTK measurements are not taken on highly reflective surfaces such as center line stripes or lane markings on roads. RTK points were taken no closer than one meter to any nearby terrain breaks such as road edges or drop offs. In addition, it is desirable to include locations that can be readily identified and occupied during subsequent field visits in support of other quality control procedures described later. Examples of identifiable locations would include manhole and other flat utility structures that have clearly indicated center points or other measurement locations. In the absence of utility structures, a PK nail can be driven into asphalt or concrete and marked with paint.

Multiple differential GPS units were used in the ground based real-time kinematic (RTK) portion of the survey. To collect accurate ground surveyed points, a GPS base unit was set up over monuments to broadcast a kinematic correction to a roving GPS unit. The ground crew used a roving unit to receive radio-relayed kinematic corrected positions from the base unit. This RTK survey allowed precise location measurement ($\sigma \leq 1.5$ cm). **Figure 2.3** shows a subset of these RTK locations.



⁴ U.S. Army Corps of Engineers , Engineer Research and Development Center Topographic Engineering Center software

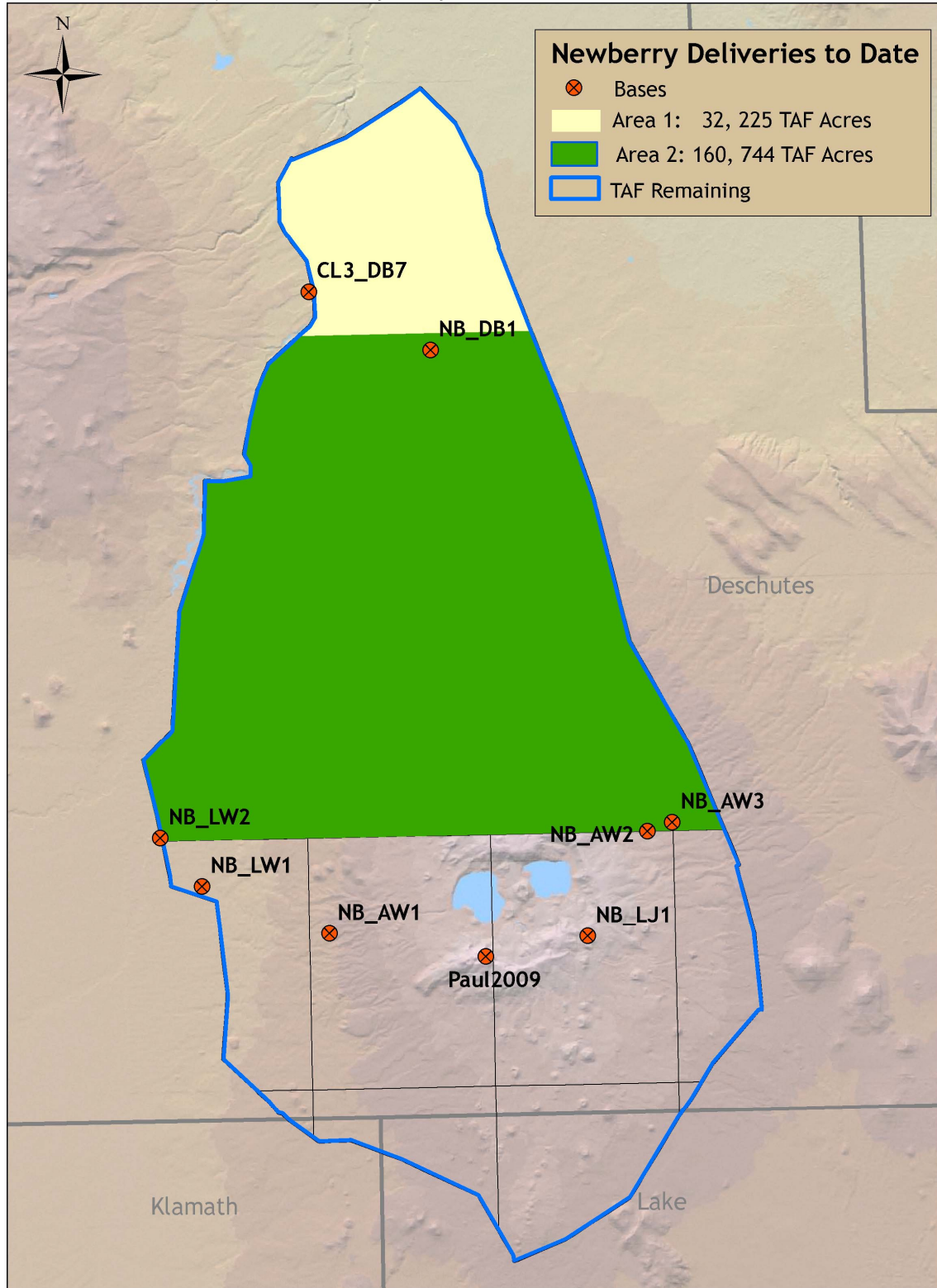
⁵ Federal Geographic Data Committee Draft Geospatial Positioning Accuracy Standards

Table 2.2. Base Station Surveyed Coordinates, (NAD83/NAVD88, OPUS corrected) used for kinematic post-processing of the aircraft GPS data for the Newberry Study Area.

Base Stations ID	Datum NAD83 (HARN)		GRS80
	Latitude (North)	Longitude (West)	Ellipsoid Height (m)
CL3_DB7	44 01 17.70633	121 22 01.03065	1160.174
NB_DB1	43 59 29.39875	121 17 02.85375	1155.739
NB_LW1	43 43 37.01802	121 26 52.42743	1271.890
NB_LW2	43 45 04.81800	121 28 33.09215	1261.725
NB_AW1	43 42 08.45361	121 21 41.57000	1505.226
NB_AW2	43 44 59.60983	121 08 33.02702	1845.624
NB_AW3	43 45 14.14298	121 07 31.60507	1790.6125
NB_LJ1	43 41 54.43972	121 11 06.24045	2109.5155
Paul2009	43 41 21.10786	121 15 17.60226	2414.443



Figure 2.2. Base stations for the Newberry Study Area.



For data delivered to date, 4,062 RTK (Real-time kinematic) points were collected in the study area. Figures 2.3 - 2.5 show detailed views of selected RTK locations for all areas.

Figure 2.3 Selected RTK point for delivery area 1; images are NAIP orthophotos.

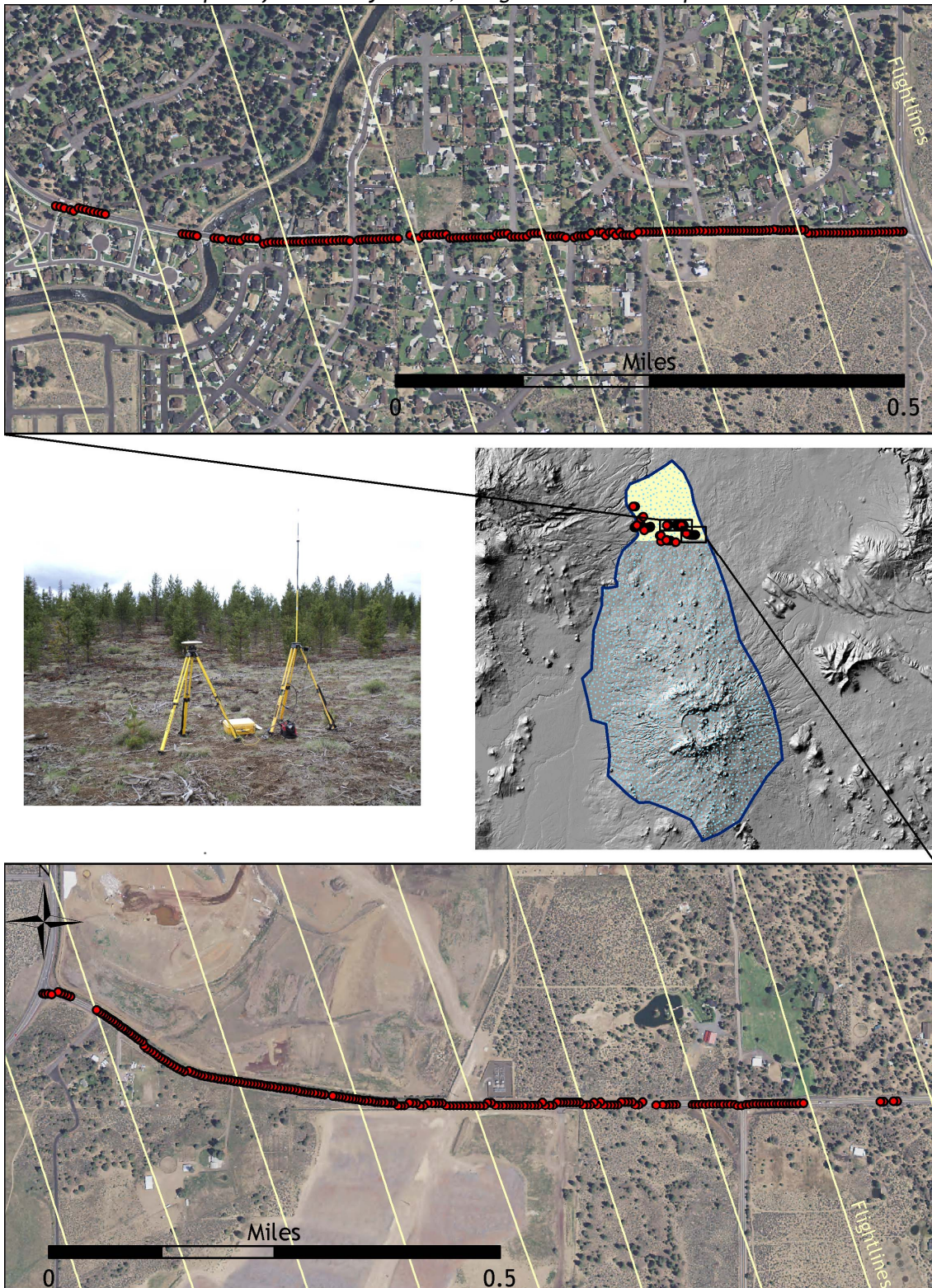


Figure 2.4. Selected RTK point locations in the study area for delivery area 2; images are NAIP orthophotos.

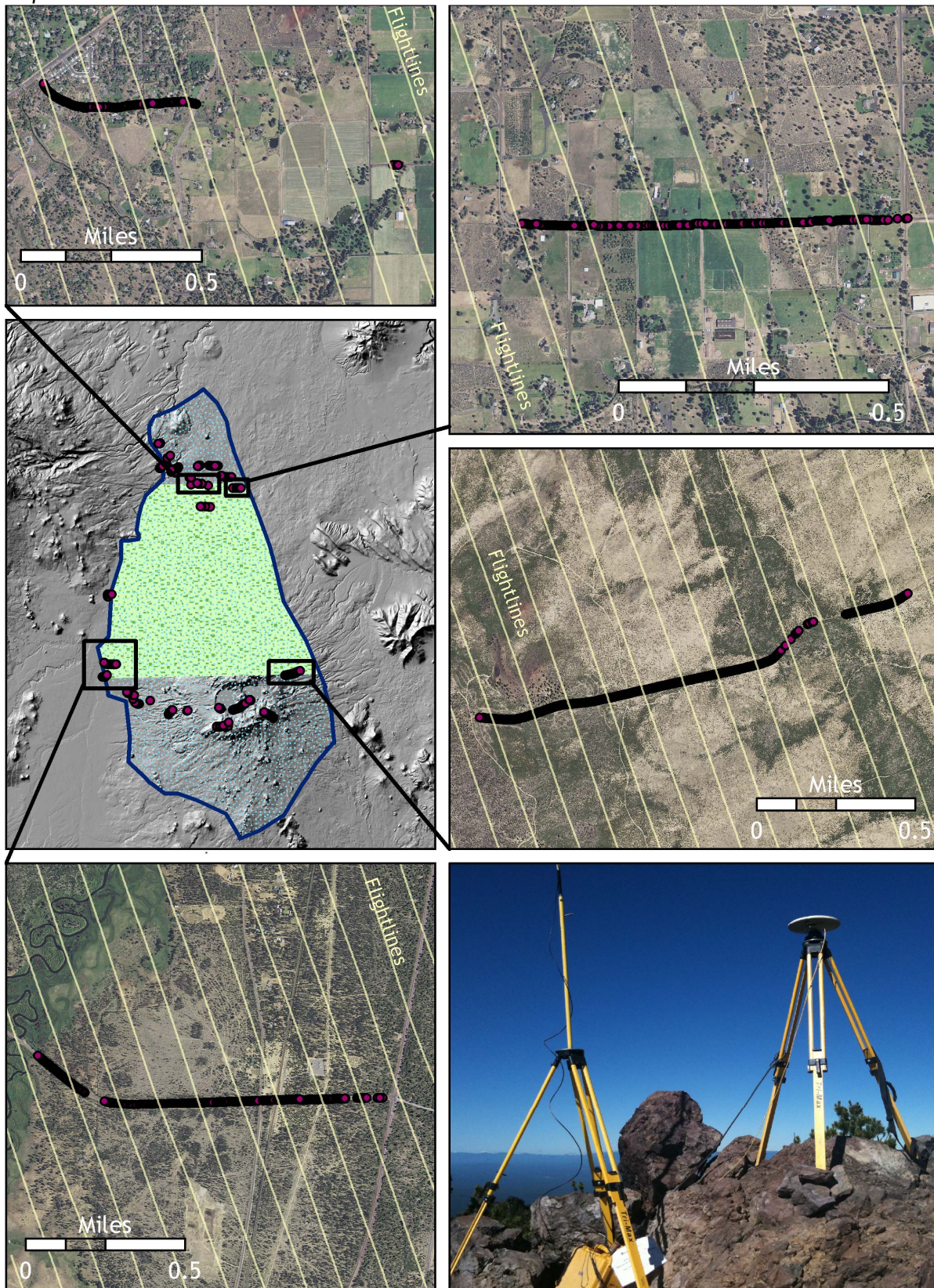
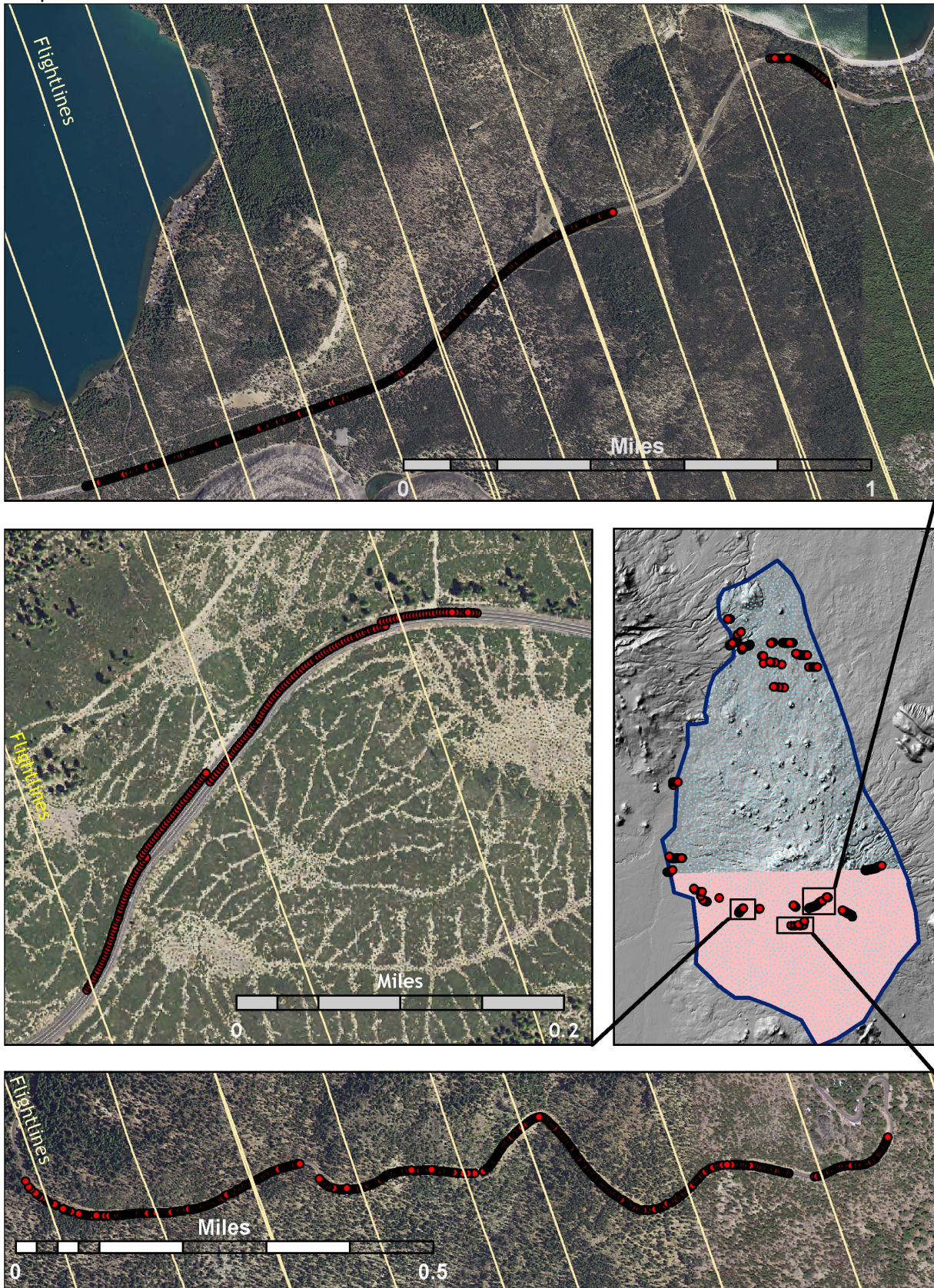


Figure 2.5. Selected RTK point locations in the study area for delivery area 3; images are NAIP orthophotos.



3. Accuracy

3.1 Relative Accuracy

Relative Accuracy Calibration Results

Relative accuracy refers to the internal consistency of the data set and is measured as the divergence between points from different flightlines within an overlapping area. Divergence is most apparent when flightlines are opposing. When the LiDAR system is well calibrated the line to line divergence is low (<10 cm). Internal consistency is affected by system attitude offsets (pitch, roll and heading), mirror flex (scale), and GPS/IMU drift.

Relative accuracy statistics are based on the comparison of 347 flightlines and over 7 billion points. Relative accuracy is reported for the portion of the study area shown in **Figure 3.1** below.

- Project Average = 0.13 ft (0.04 m)
- Median Relative Accuracy = 0.13 ft (0.04 m)
- 1 σ Relative Accuracy = 0.14 ft (0.04m)
- 2 σ Relative Accuracy = 0.19 ft (0.06 m)

Figure 3.1. Relative Accuracy Covered Area.

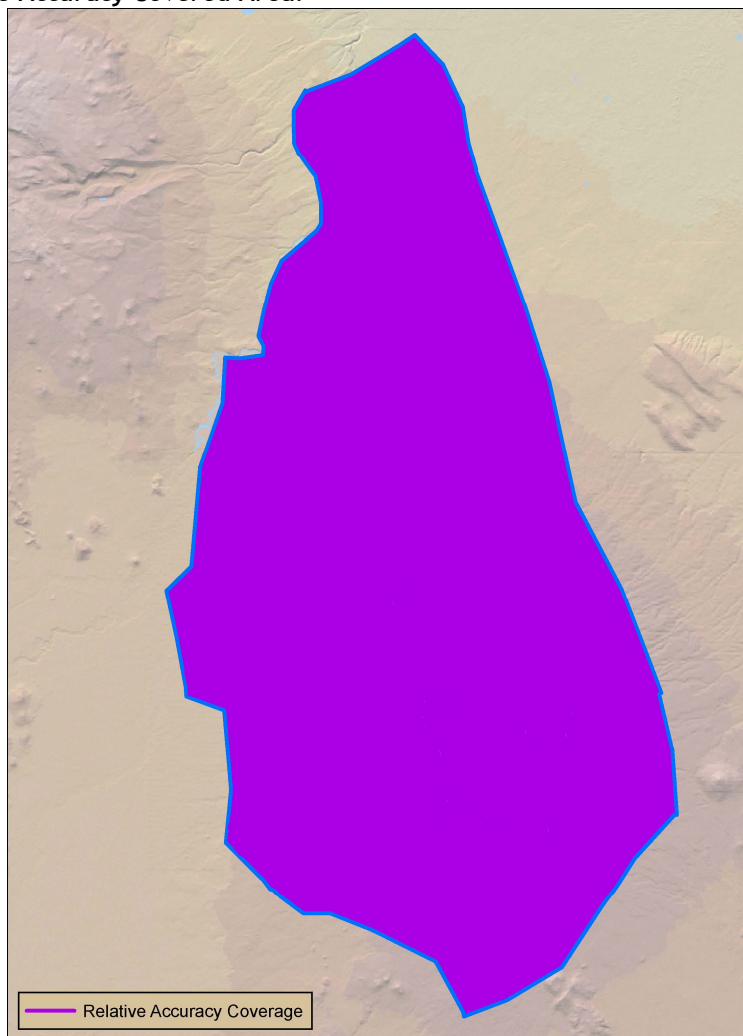


Figure 3.2. Statistical relative accuracies, non slope-adjusted.

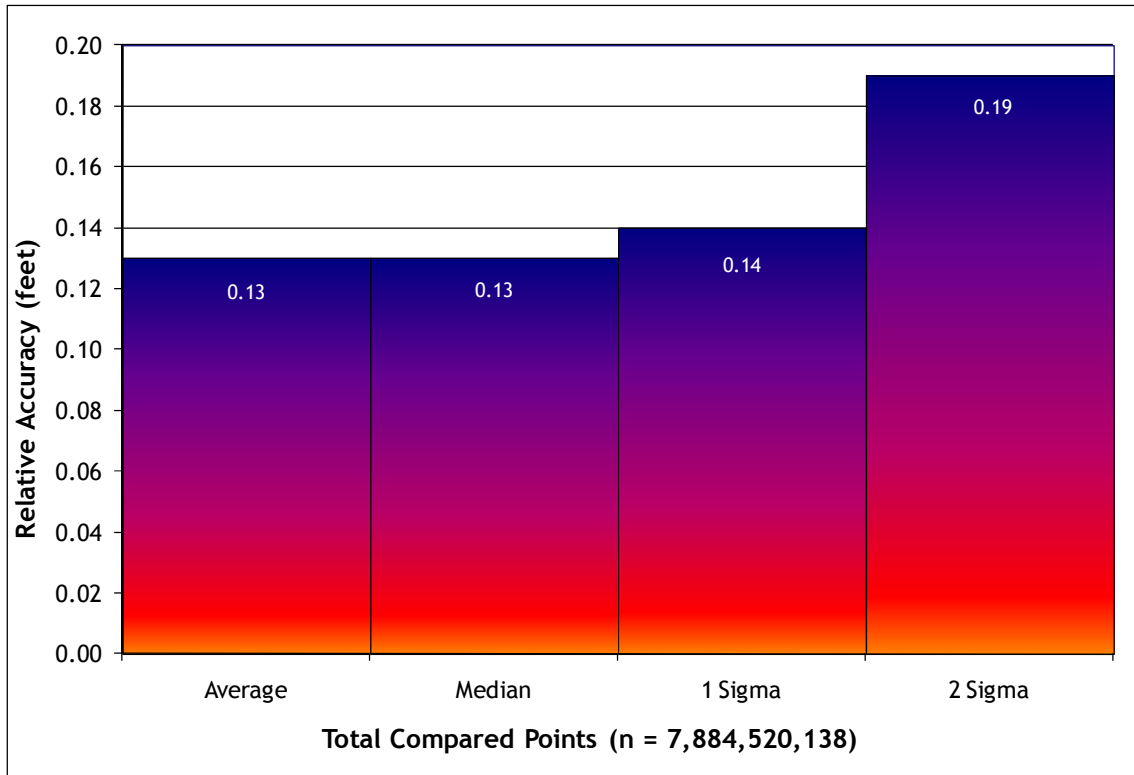
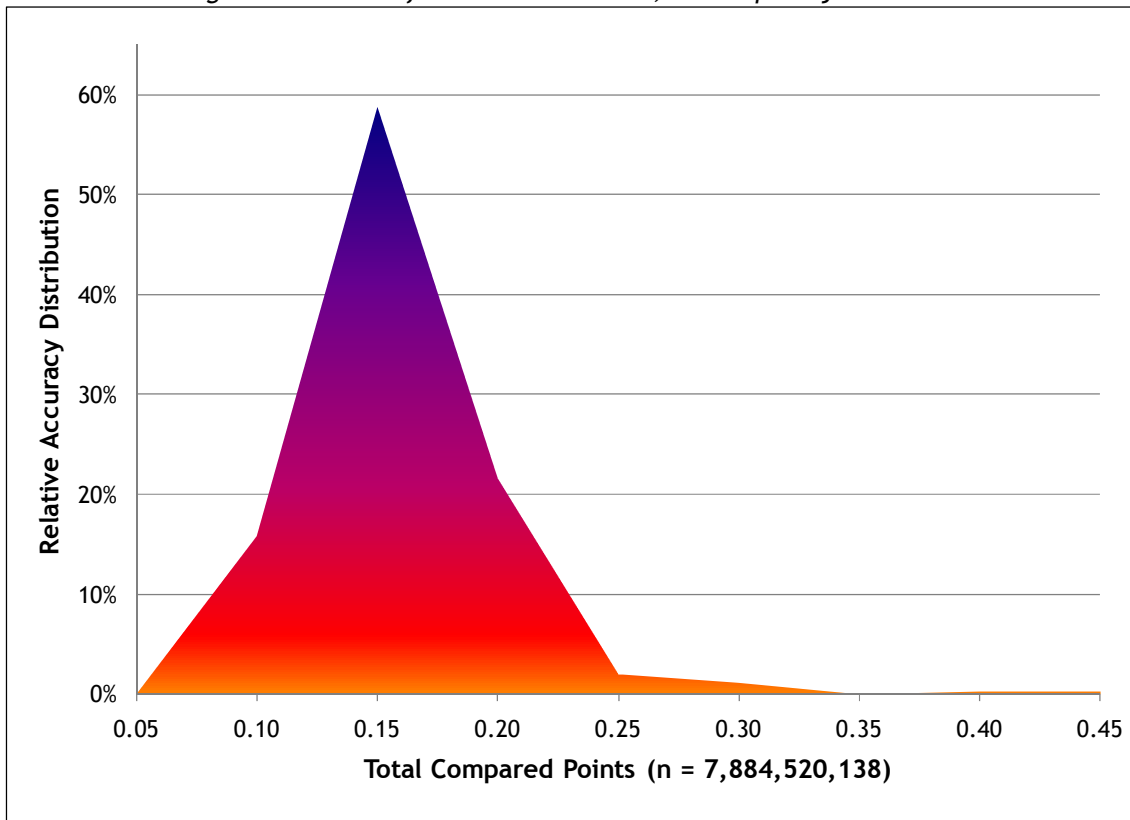


Figure 3.3. Percentage distribution of relative accuracies, non slope-adjusted.



3.2 Absolute Accuracy

Absolute accuracy compares known RTK ground survey points to the closest laser point. For the Newberry Study Area, 4,062 RTK points were collected for data in the study area. Absolute accuracy is reported for the portion of the study area shown in **Figure 3.4** and reported in **Table 3.1** below. Histogram and absolute deviation statistics are reported in **Figures 3.5** and **3.6**.

Table 3.1. Absolute Accuracy - Deviation between laser points and RTK survey points.

Sample Size (n): 4,062	
Root Mean Square Error (RMSE): 0.12 ft (0.04m)	
Standard Deviations	Deviations
1 sigma (σ): 0.12 ft (0.04 m)	Minimum Δz : -0.38 ft (-0.12 m)
2 sigma (σ): 0.23 ft (0.07 m)	Maximum Δz : 0.26 ft (0.08 m)
	Average Δz : 0.10 ft (0.03 m)

Figure 3.4. Absolute Accuracy Covered Area.

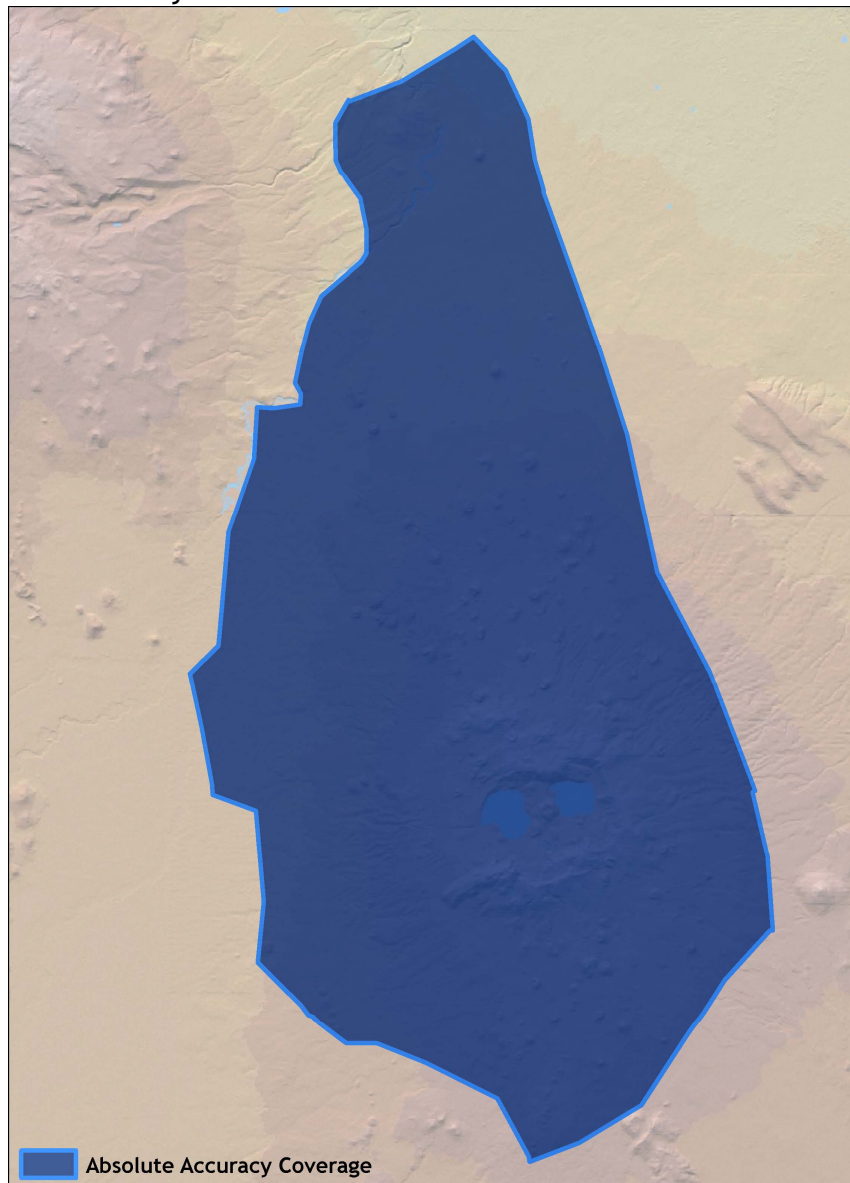


Figure 3.5. Newberry Study Area histogram statistics

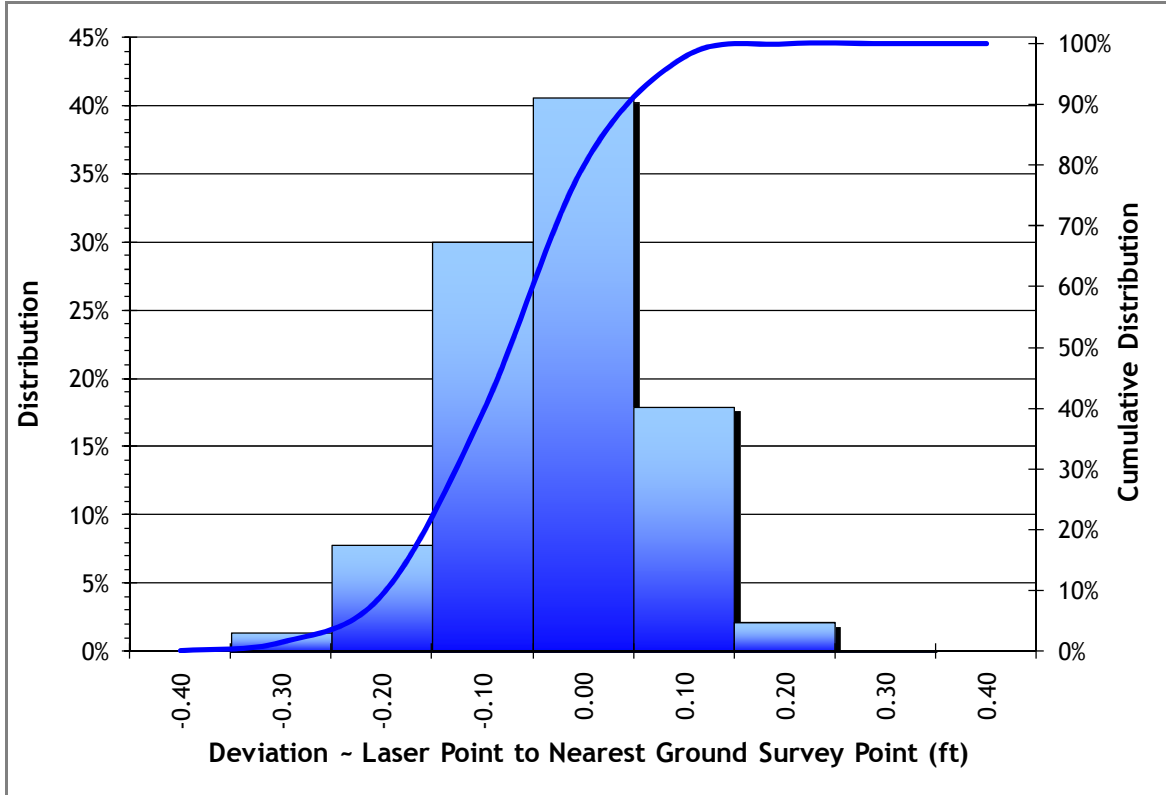
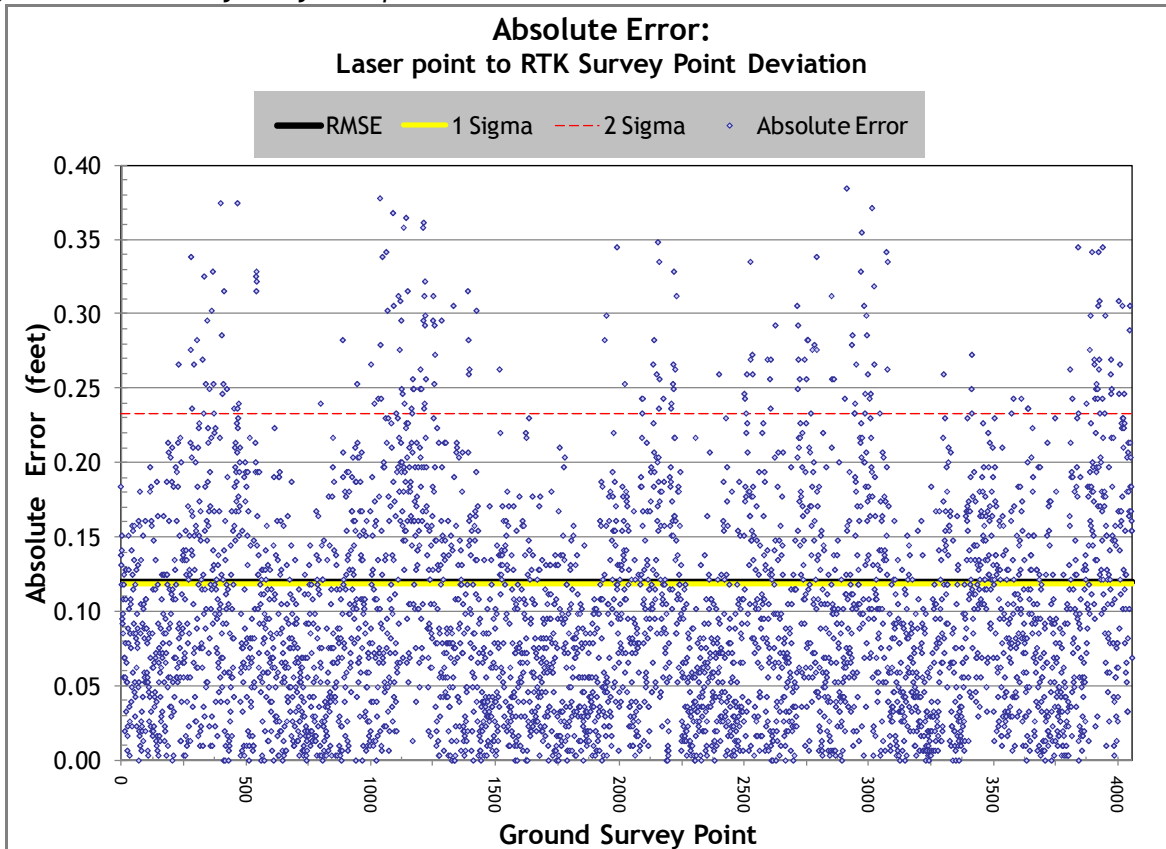


Figure 3.6. Newberry Study Area point absolute deviation statistics.



4. Data Density/Resolution

4.1 Density Statistics

Some types of surfaces (i.e. dense vegetation or water) may return fewer pulses than the laser originally emitted. Therefore, the delivered density can be less than the native density and vary according to terrain, land cover and water bodies. Density histograms and maps (Figures 4.1 - 4.4) have been calculated based on first return laser point density and ground-classified laser point density.

Table 4.1. Average density statistics for Newberry Study Area data delivered to date.

Average Pulse Density (per square ft)	Average Pulse Density (per square m)	Average Ground Density (per square ft)	Average Ground Density (per square m)
0.80	8.57	.22	2.41

Figure 4.1. Histogram of first return laser point density for data delivered to date.

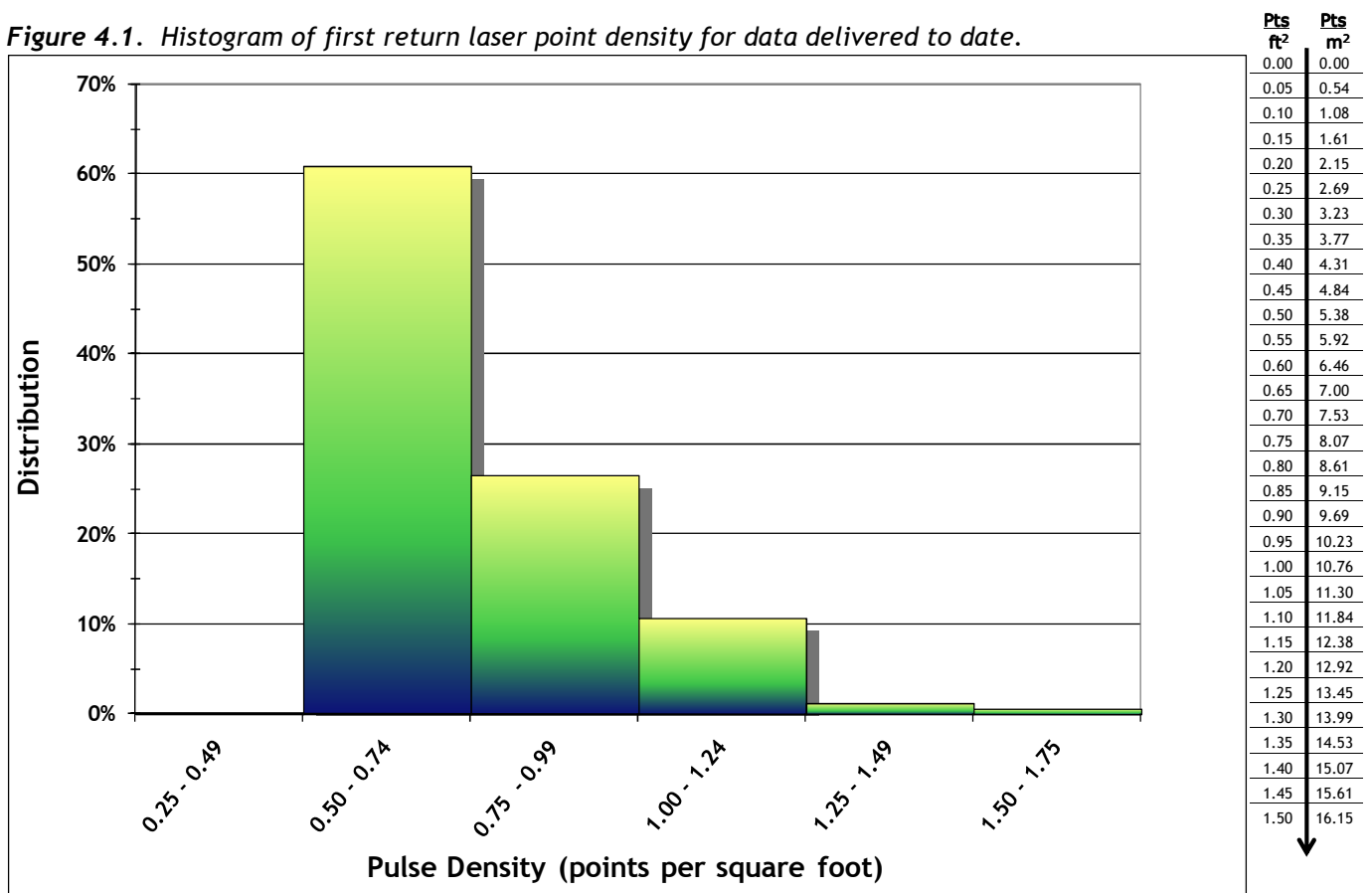
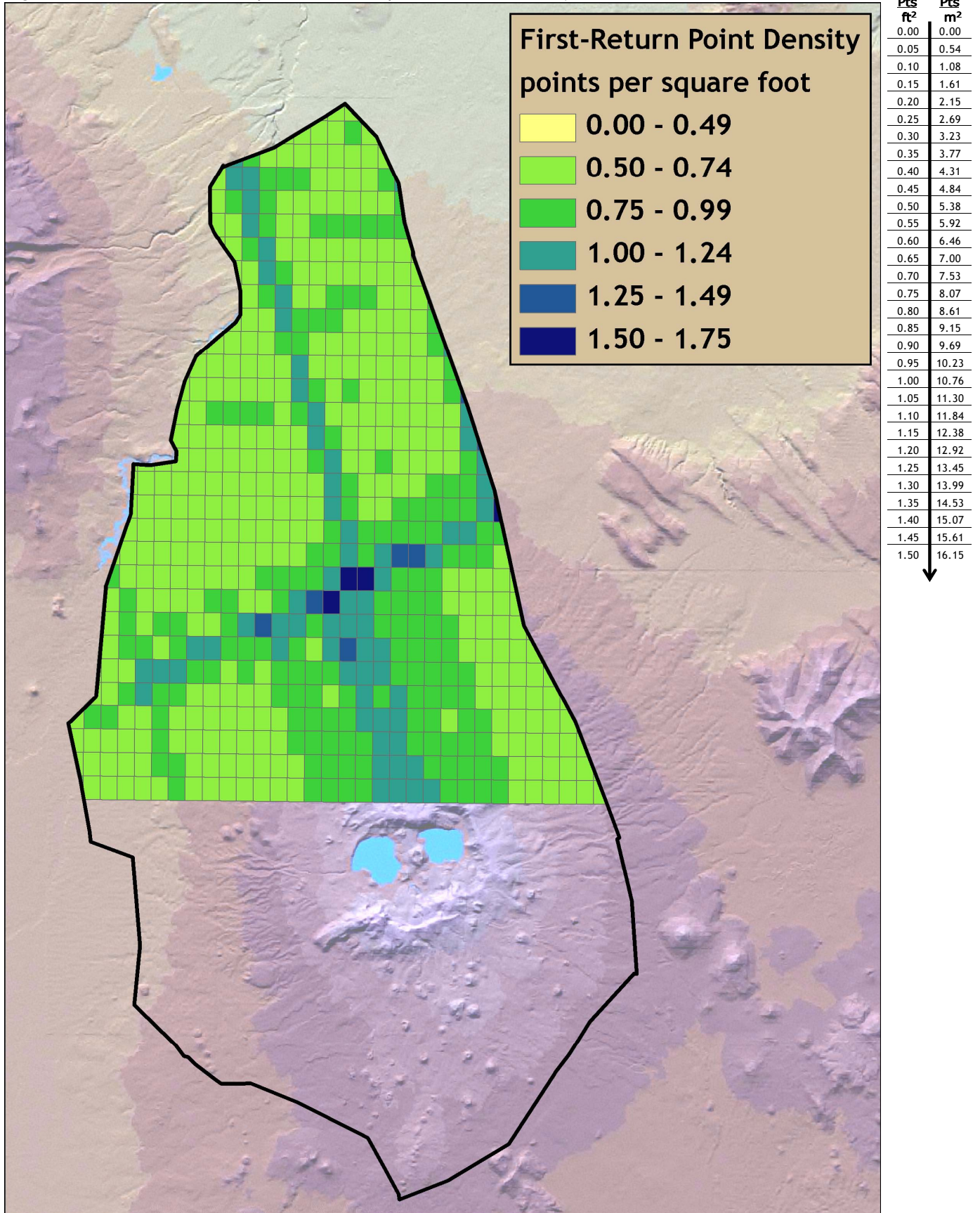
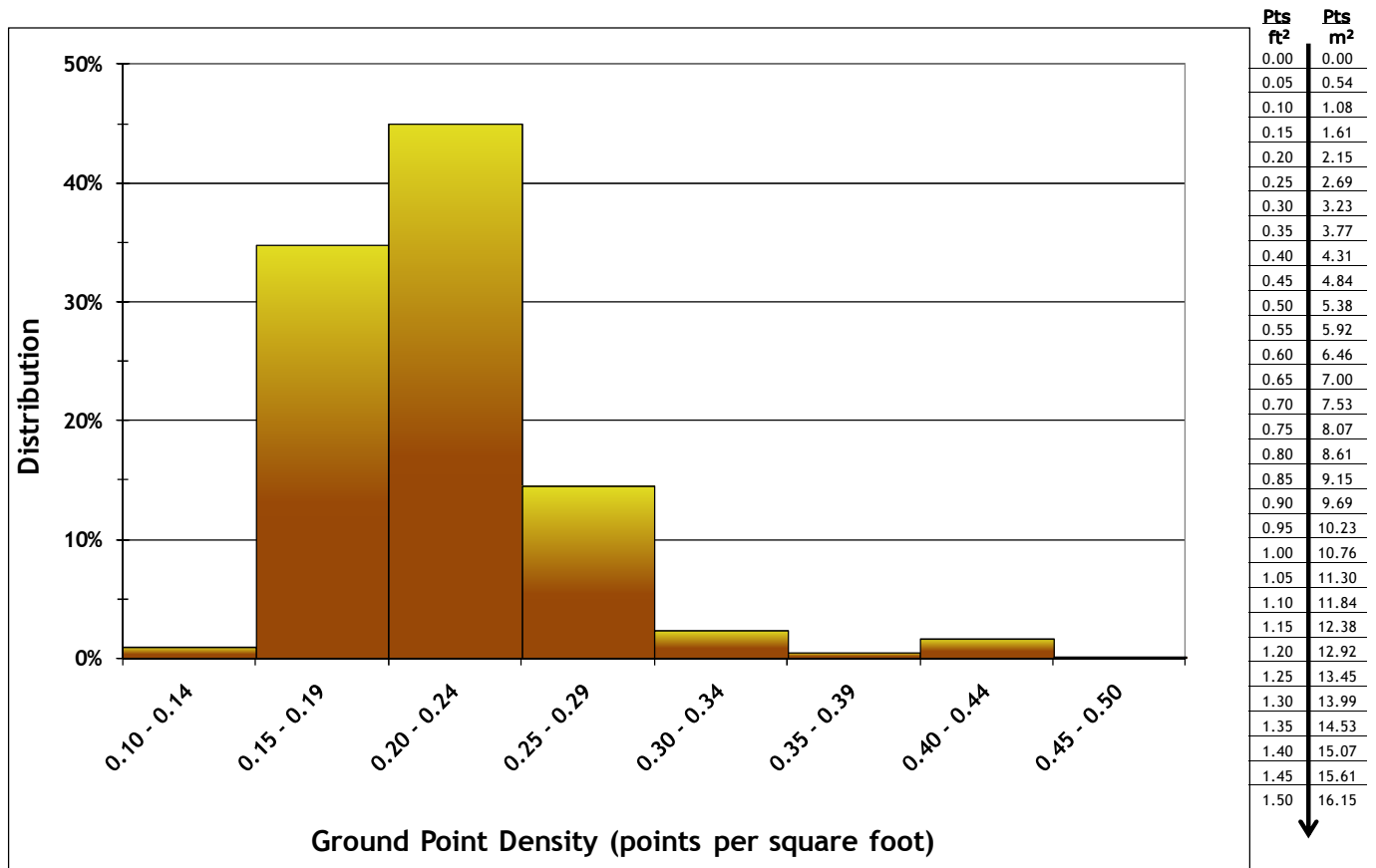


Figure 4.2. First return laser point densities per 0.75' USGS Quad for data delivered to date.



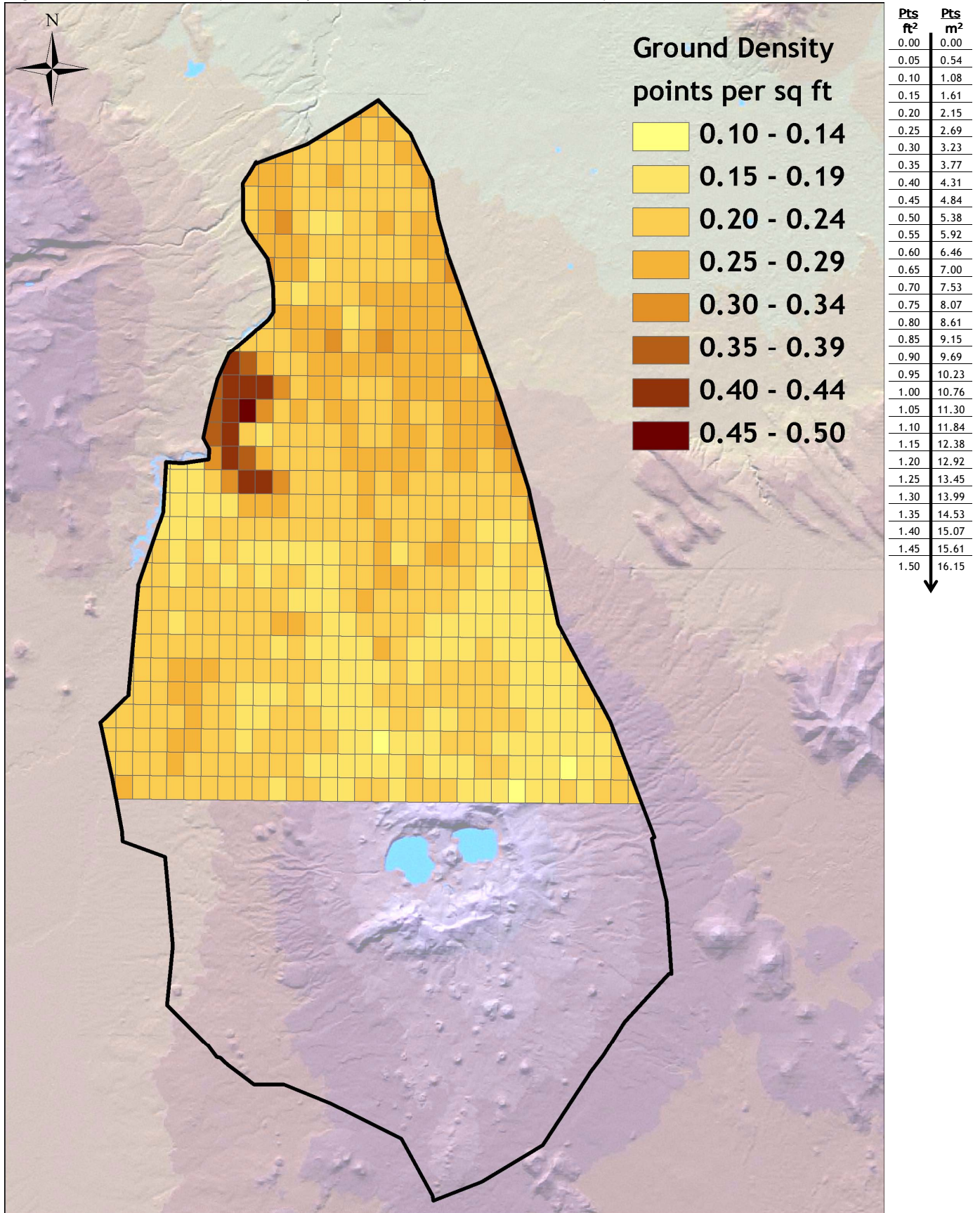
Ground classifications were derived from ground surface modeling. Classifications were performed by reseeded of the ground model where it was determined that the ground model failed, usually under dense vegetation and/or at breaks in terrain, steep slopes and at bin boundaries.

Figure 4.3. Histogram of ground-classified laser point density for data delivered to date.



Pts ft ²	Pts m ²
0.00	0.00
0.05	0.54
0.10	1.08
0.15	1.61
0.20	2.15
0.25	2.69
0.30	3.23
0.35	3.77
0.40	4.31
0.45	4.84
0.50	5.38
0.55	5.92
0.60	6.46
0.65	7.00
0.70	7.53
0.75	8.07
0.80	8.61
0.85	9.15
0.90	9.69
0.95	10.23
1.00	10.76
1.05	11.30
1.10	11.84
1.15	12.38
1.20	12.92
1.25	13.45
1.30	13.99
1.35	14.53
1.40	15.07
1.45	15.61
1.50	16.15

Figure 4.4. Ground-classified laser point density per 0.75' USGS Quad for data delivered to date.



5. Selected Imagery

Figure 5.1. Lava Butte off Hwy 97 in Deschutes National Forest, twelve miles south of Bend, Oregon. Image is a three dimensional point cloud with RGB values extracted from a NAIP orthophoto.



Figure 5.2. Logging area near Paulina Creek off Paulina-East Lake Rd, northeast of La Pine, Oregon. *Top image facing east; Bottom image facing north. Image is a three dimensional point cloud with RGB values extracted from a NAIP orthophoto.*



5.3. Oblique view of the Deshutes River in Bend, Oregon. Image is a three dimensional point cloud with RGB values extracted from a NAIP orthophoto.



Figure 5.4. View from the Northwest of Pilot Butte near Bend, Oregon. Image is a three dimensional LiDAR point cloud with RGB values extracted from a NAIP orthophoto.



Figure 5.5. View from the east of an area north of Bend, Oregon along the Deschutes River. Image is a three dimensional LiDAR point cloud with RGB values extracted from a NAIP orthophoto.

