

# LIDAR ACQUISITION and CONTROL SURVEY REPORT

Subcontract Agreement: S/C-SCDNR-001  
Task order: 1 – Pickens, Oconee, Anderson County Lidar

*Prepared for*



**Dewberry**

*Prepared by*



**TOWILL** | Surveying, Mapping  
and GIS Services

July, 2011



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## EXECUTIVE SUMMARY

Under contract with Dewberry and Davis (Dewberry), Towill acquired Lidar data (nominal point spacing of 1.0 meters) within the 100-meter buffered boundary of Pickens, Oconee, and Anderson Counties, South Carolina. The data were acquired in 15 missions over the course of 13 days in March, 2011.

As part of the campaign, a 3-dimensional primary survey network consisting of local South Carolina Geodetic Survey VRS and North Carolina CORS stations and a semi-permanent base station point was observed to establish the basis of control for the Lidar data. The selected horizontal and vertical datums upon which the Lidar data are processed are NAD83(NSRS2007); epoch of 2007.0 and the North American Vertical Datum of 1988 (NAVD88) as realized by the reported coordinates and ellipsoid heights of the VRS and CORS and the application of GEOID09. This primary network satisfied U.S. Federal Geodetic Control Subcommittee (FGCS) standards for Order B geodetic GPS surveys (8mm + 1 part per million).

In addition, 40 check points and 6 existing NGS bench marks were surveyed to demonstrate the absolute accuracy of the Lidar data. The root-meant-square (RMS) of the differences between the check point surveyed elevations and the Lidar-derived surface model was 0.064 meters (0.21 foot).

Rigorous Lidar sensor calibration and quality control procedures were applied during the course of the campaign. Calibration passes were flown at the beginning and end of each mission and analyzed to verify the performance of the sensor and to make small adjustments to the final processing parameters.

In summary, all of the project's geodetic surveying and mapping goals were achieved. This report provides detailed documentation of all aspects of the work.

**Towill, Inc.**



Keith Kirkby  
*Geodetic Engineer*  
July, 2011

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## 1. INTRODUCTION

This document provides a comprehensive overview of the Lidar acquisition campaign to acquire Lidar data of Pickens, Oconee, and Anderson Counties, South Carolina. The report describes the field survey associated with establishing base stations to support the airborne GPS (AGPS) component of the campaign, Lidar system calibration, Lidar data post-processing, and QA/QC of the data.

The Lidar acquisition was completed in a total of 15 separate flight missions between March 13th and March 25th, 2011 inclusive. Flight operations were staged out of the Anderson Regional Airport located in Anderson, South Carolina.

All data acquisition field work, data post-processing and quality analysis was completed by Towill personnel. The components of the campaign include:

- Establishing and surveying AGPS base stations, control, and check points;
- Verifying Lidar system calibration and post-processing parameters;
- Airborne GPS (AGPS) and IMU data post-processing;
- Pre and post-mission control surface overflight data analysis;
- Surface check point survey analysis;

### 1.1 Points of Contact

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## 2. Control Survey and Datums

### 2.1 Introduction

The underlying basis of the Lidar survey campaign are the published coordinate and elevation values of the South Carolina Geodetic Survey (SCGS) active VRS network stations. The coordinates and elevations were provided to Towill directly from the SCGS.

Towill observed a primary geodetic network consisting of the local VRS stations and a base station point established at the airport of operations to support the airborne GPS component of the Lidar campaign thereby ensuring a consistent horizontal and vertical datum realization across the entire extent of the project area (see Figure 1). The network consists of 4 SCVRS, 1 North Carolina CORS, and 1 designated GPS base station.

Independent CORS and VRS observation data were downloaded in the days leading up to the start of the acquisition campaign to establish the relative baseline observations between the stations. The remaining baselines were observed using the data from the base station occupations during acquisition and additional VRS data from those respective days.

Final coordinates and elevations of the base station locations were established via a 3-dimensional network adjustment constrained to the published horizontal coordinates and ellipsoid heights of the SCVRS (as described in section 2.5, below).

In addition, 40 check points and 6 existing NGS bench marks were surveyed during the Lidar acquisition. These locations were surveyed with respect to the operating base stations and nearby VRS and base stations.

### 2.2 Project Survey Datums

The horizontal datum for this project is NAD83(NSRS2007); epoch of 2007.0. The datum is realized by the horizontal coordinates reported by the South Carolina Geodetic Survey of the VRS stations and the published coordinates of the North Carolina CORS (see Table 2).

The vertical datum for this project is NAVD88. The datum is realized by the ellipsoid heights of the same VRS and CORS stations reported from SCGS and the application of the geoid model GEOID09.

### 2.3 Field Equipment and Procedures

All GPS observations were accomplished using Trimble Navigation R7 dual frequency GPS receivers and accompanying Trimble Zephyr Geodetic antennae. Relative static surveying techniques were used for all baseline observations. Instrument heights were measured twice in units of feet and meters and the values reduced and compared in the field prior to leaving each station.

In general, base station data were logged for the duration of the acquisition on any given day (typically 4 to 12 hours) and check point data were logged for a minimum of 30 minutes and as much as 90 minutes depending on proximity to operating base stations and/or VRS station.

### 2.4 Primary Survey Network and Adjustment

Observed relative GPS baselines were processed in Trimble Business Center. All processed observations consist of quasi-independent baselines (i.e. in accordance with the “ $n-1$  baselines” rule where  $n$  = number of receivers in a given ‘session’). The International GPS Service for Geodynamics (IGS) rapid precise orbits (igr) were used in the processing of all baseline vectors. The ‘igr’ orbits are published with a latency of approximately 30 hours. These orbits are globally accurate to within ~5cm and are

particularly important when processing long baselines.

The absolute horizontal and vertical GPS loop misclosures for the primary network are presented in Table 1. The spatial misclosures in parts per million (ppm) are also listed. All loops comprise quasi-independent baselines from at least two ‘sessions’.

The primary survey network, consisting of 6 points and 11 baselines, was designed to provide a basis for the Lidar control (i.e. AGPS base stations) and establishing additional quality control points for this project. A minimally constrained adjustment was executed to verify the internal integrity of the network, establish *a priori* weights for the GPS observations, and judge the absolute fit of the constraints.

The GPS baselines vector components were adjusted using Microsearch™ GEOLAB 2001 (version 2001.9.20.0). *A priori* weights for the observations were based on the scaled variance–covariance sub-matrices estimated by the Trimble Business Center software. In the resulting adjustment, the estimated variance factor ( $\hat{\sigma}_o^2 = 1.003$ ) passed the  $\chi^2$ -test indicating appropriate *a priori* estimates of the accuracy of the GPS baseline vectors. None of the 33 vector component residuals or associated standardized residuals were flagged for possible rejection under the  $\tau$ -max test at the 95 percent level of confidence. The relative horizontal accuracy of the network can be assessed by reviewing the relative 95 percent confidence regions (ellipses) of the adjustment. All station pairings meet the Federal Geodetic Control Subcommittee (FGCS) relative positioning standard for Order B surveys (8mm + 1ppm).

In a second, fully constrained adjustment, the NAD83 latitude, longitude and ellipsoid height of the 4 SCVRS stations and 1 NC CORS were held as *weighted* constraints (see Table 2). The estimated variance factor ( $\hat{\sigma}_o^2 = 0.8691$ ) indicates that the network is not being unduly distorted by the imposition of the constraints and is maintaining its internal integrity. The adjustment yields coordinates on the NAD83 and orthometric elevations relative to NAVD88 via the GEOID09 geoidal model. These coordinates serve as the control for the post-processing of all Lidar data and subsequent products derived from the Lidar data. See Appendix II for the primary constrained adjustment listing.

A third and final adjustment was run to incorporate the check point and bench mark observations. The adjusted coordinate values from the fully constrained adjustment were held as *fixed* constraints to derive final coordinate and elevation values of the check points. Appendix I tabulates the final adjusted coordinate and elevation values of all surveyed points.



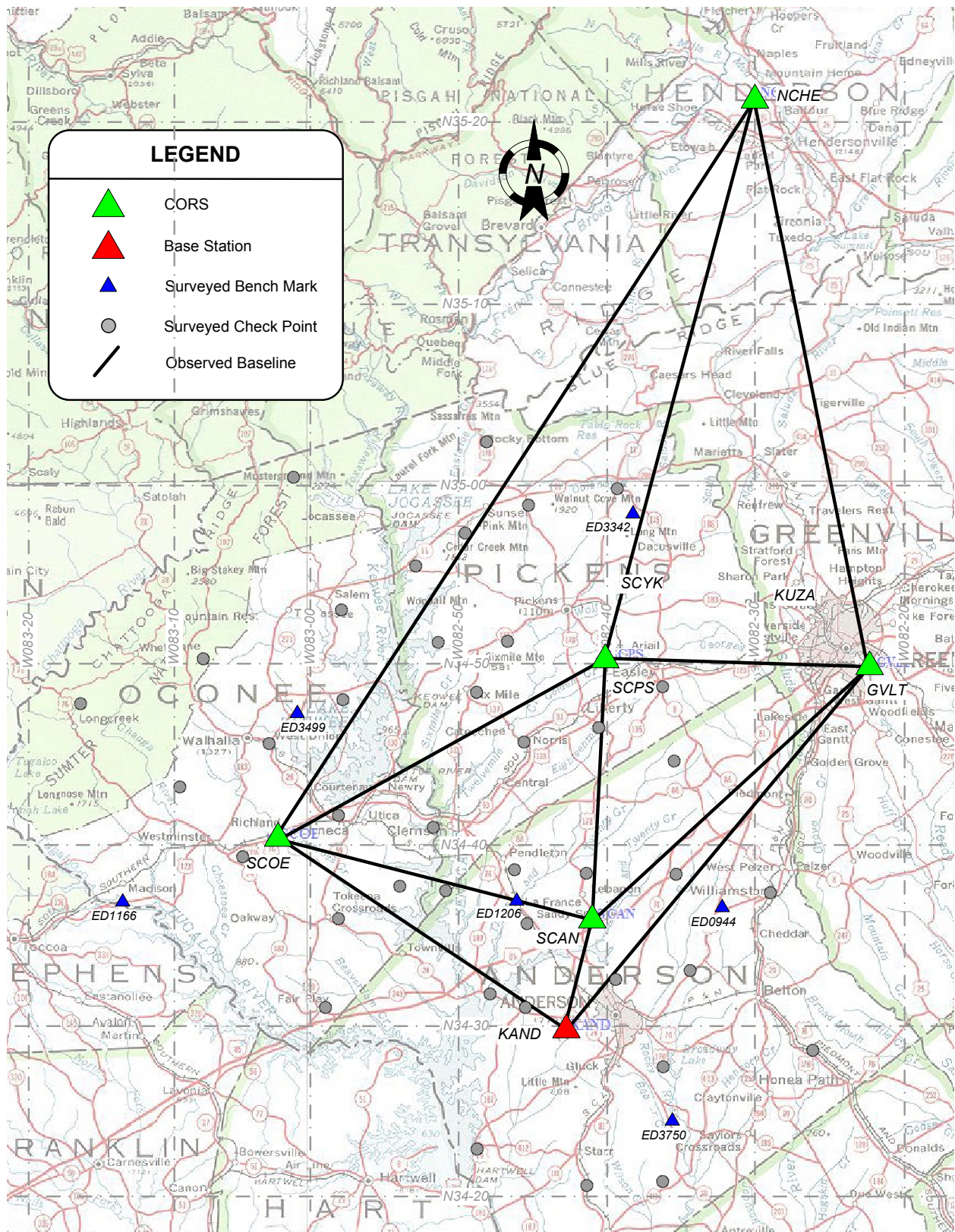


Figure 1. Primary Survey Network Diagram

Table 1.  
Absolute Loop Misclosures

Loop	Horizontal Misclosure [mm]	Vertical Misclosure [mm]	Loop Length [meters]	PPM
NCHE - GVLТ - SCPS - NCHE	3	38	146706	0.26
SCPS - GVLТ - SCAN - SCPS	4	65	92699	0.70
GVLТ - KAND - SCAN - GVLТ	6	32	98724	0.33
SCPS - SCAN - KAND - SCOE - SCPS	4	11	112180	0.11
NCHE - SCPS - SCOE - NCHE	4	39	188162	0.21

Table 2.  
Primary Network Adjustment Weighted Constraints

Horizontal Datum: NAD83(NSRS2007)  
Epoch: 2007.0  
Linear Unit: International Meter

Point	Source	Latitude			Longitude			Ellipsoid Height	Adjustment Residuals			
		°	'	"	°	'	"		Lat.	Lng.		
SCPS	SCGS	N	34	50	13.06816	W	82	40	15.72001	300.783	-0.002	0.004
SCAN	SCGS	N	34	35	48.45792	W	82	41	00.47566	229.892	-0.012	0.000
SCOE	SCGS	N	34	40	19.81527	W	83	02	15.01590	232.457	0.004	0.007
GVLТ	SCGS	N	34	49	46.94102	W	82	22	15.37249	250.260	0.003	-0.007
NCHE	USGS	N	35	21	21.89186	W	82	30	03.99744	654.460	0.006	-0.004

Notes: Ellipsoid heights are to the Antenna Reference Point (ARP) of the VRS and CORS

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### 3. LIDAR DATA ACQUISITION AND PROCESSING

#### 3.1 Introduction

Following is an overview description of the procedures applied in this Lidar campaign from acquisition to final processed data. Figure 2 illustrates the general flow of the data through the multiple processes required to generate the Lidar point cloud in the 'LAS' version 1.2 format.

#### 3.2 Data Acquisition

The Lidar data acquisition was completed within 15 lifts, or missions. All missions originated and/or terminated at the Anderson Regional Airport. A GPS base stations was operating at the airport during every lift. The South Carolina Geodetic Survey was contacted prior to the start of the Lidar acquisition to arrange for 1 Hertz data logging of several of the VRS stations included in the primary survey network. The data from these stations were downloaded and applied in the post-processing of the kinematic AGPS data.

Figure 3 illustrates the general flight plan. The target flying height of the flight lines was 1,500 meters above mean terrain. Figure 4 summarizes the general acquisition parameters applied project-wide.

Kinematic GPS and Inertial Measurement Unit (IMU) data were acquired by the Applanix POS Inertial Navigation System during the missions. The post-processed POS data results in a 200 Hertz, 6-parameter aircraft trajectory (x, y, z, roll, pitch, yaw).

The Airborne GPS (AGPS) and IMU data were processed immediately following each mission. In addition, a sample of the Lidar data was post-processed at the completion of the missions and the data was reviewed to ensure correct system operation and data coverage.

#### 3.3 Airborne GPS Processing

The quality of the Airborne GPS data represents a significant component of the overall error budget with respect to the accuracy of the Lidar data. It is important to exercise vigilance in the validation of the integrity of the AGPS solution. This effort begins prior to acquisition with careful mission planning to identify periods of the day during which satellite availability and/or geometry may not be conducive to an acceptable solution. Data acquisition is generally scheduled around these periods (other constraints such as airspace restrictions, daylight conditions and weather notwithstanding).

The kinematic AGPS data was post-processed using Novatel, Inc.'s Grafnav version 8.30 software, the *de facto* kinematic GPS post-processing package in the airborne remote sensing industry. Data is post-processed forward and backward in time exploiting the software's robust Kinematic Ambiguity Resolution and Multi-Baseline features to mitigate ambiguity drift and minimize poor data as a result of satellite loss of lock (see example in Figure 5). Figure 6 illustrates the comparison of the forward and reverse solutions of the post-processed GPS data for one of the missions. The plot exhibits a very satisfactory solution and it represents the standard results achieved in the post-processing of all missions.



### LIDAR Data Post-Processing Data Flow

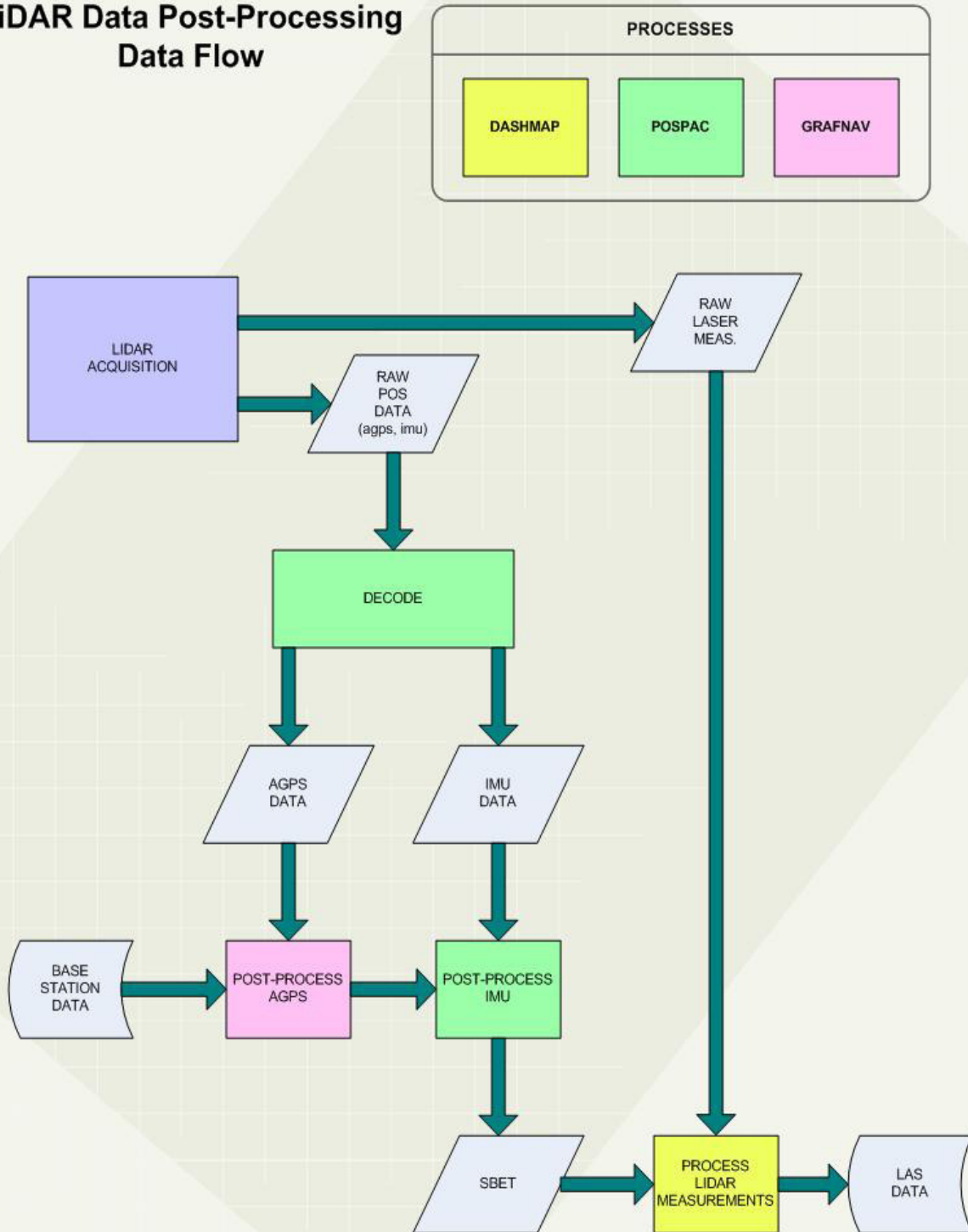


Figure 2. Lidar Data and Post-Processing Data Flow



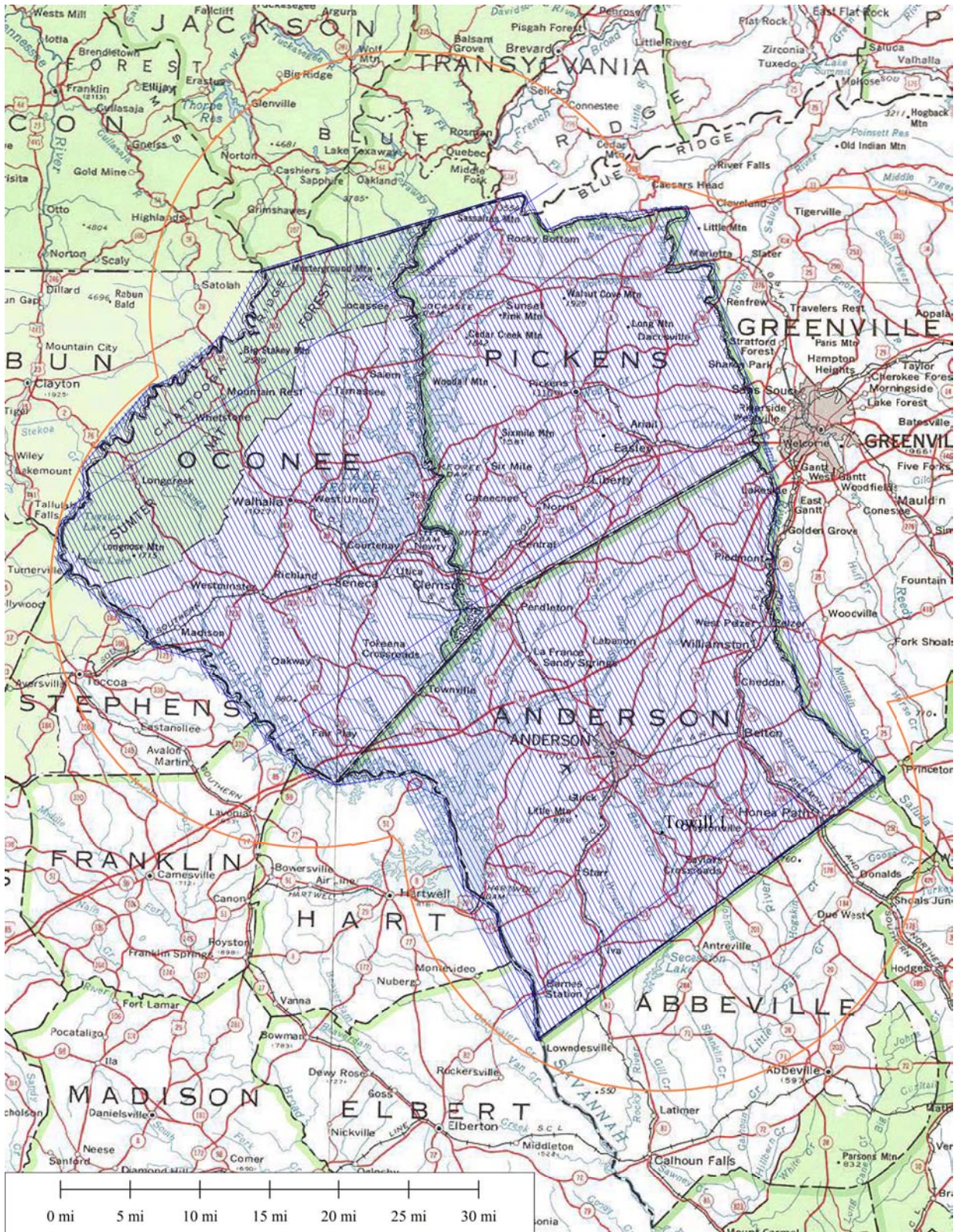


Figure 3. Lidar Flight Layout



Flight Profile		LIDAR Settings	
Altitude (m AGL)	1500	System PRF (kHz)	70
Unaided NOHD (m)	163	Scan Freq (Hz)	38
Pass Heading (deg)	332	Scan Angle +/-	20
Overlap (%)	50	Scan Offset	0
Speed (kts)	150	Desired Res (m)	1.097
Turn Time (min)	5	CT Res	1.186
Passes	154	DT Res	1.015
Pass Spacing (m)	545.36	PPM^2	0.83
Min DEM Altitude	0	Scan Cutoff (deg)	0.02
Max DEM Altitude	0	Swath (m)	1090.72

Figure 4. General Project Lidar Acquisition Parameters

### 3.4 IMU Processing and Best Estimated Trajectory

The post-processed AGPS trajectory is combined with the raw 200 Hertz IMU observations in a loosely-coupled Kalman filter-based processing algorithm to produce the final high-frequency Smoothed Best Estimated Trajectory (SBET). Applanix's POSpac software, version 4.4, is employed in this process.

Given a good quality AGPS solution and clean, gap-free IMU data, this process generally runs very smoothly. The field procedure includes several minutes of static GPS and IMU data collection prior to departure to allow sufficient time for the IMU to acquire a fine local level. The data is acquired in duplicate in real-time to ensure a high-quality record set. The IMU processing was clean and consistent for all missions during this campaign.

The final, high-frequency SBET is the source of absolute geo-referencing of the post-processed Lidar point cloud. The SBET is introduced into the final phase of the Lidar data processing.

### 3.5 LIDAR Point Cloud Processing

Final Lidar data processing is accomplished using Optech's DASHMap software, version 5.20. The decoded raw laser observations (ranges, intensities, and mirror angles) and the final processed SBET are combined within DASHMap to compute the final 3-dimensional coordinates of the return(s) of each laser pulse.

Based on the daily calibration analysis, described in the following section of this report, several of the Lidar parameters are slightly adjusted within DASHMap on a mission-by-mission basis.

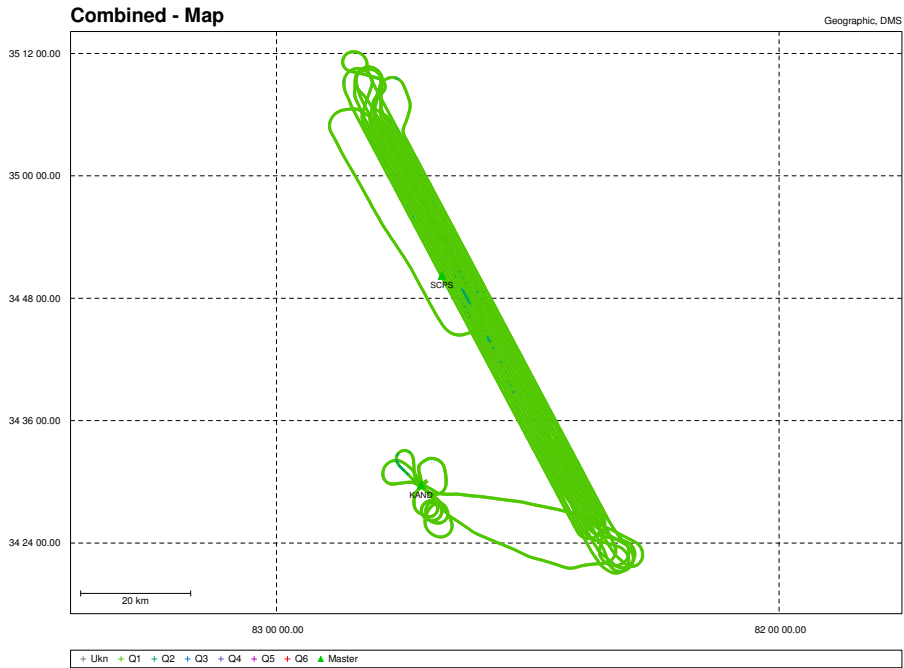
The calibrated data is output in LAS format, version 1.2 with "adjusted" GPS times (defined as GPS seconds of the week minus  $1 \times 10^{-9}$  seconds).





Project: Day072B

GrafNav v8.30.2105



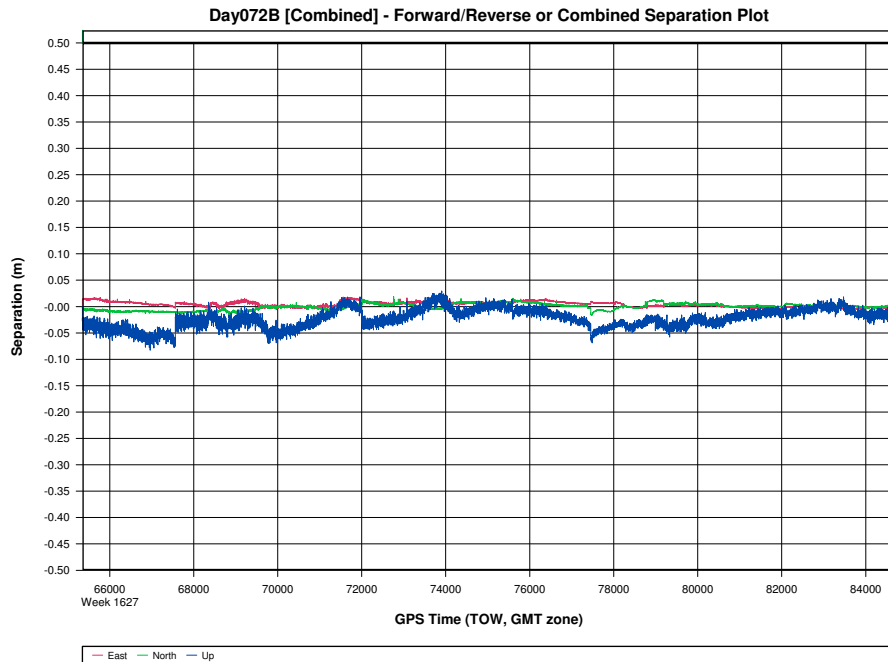
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Figure 5. Post-Processed GrafNAV Solution - Lift 072B

Project: Day072B

GrafNav v8.30.2105



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Figure 6. Forward vs. Reverse Post-Processed Solution - Lift 072B

## 4. LIDAR CALIBRATION AND QC

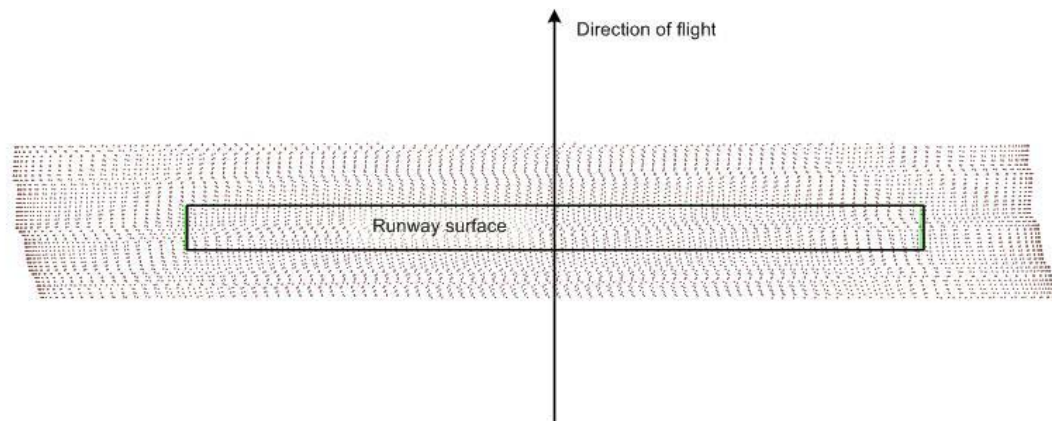
### 4.1 Introduction

The Optech Orion M200 Lidar system is subject to a regular maintenance and calibration schedule. The intent of periodic calibration is to monitor and validate components of the overall error budget including IMU boresight and performance, mirror angle readings and pulse gate timing. Several of these parameters can vary during and between missions due to changes in ambient meteorological conditions, different flying heights above ground, and different acquisition variables. As such, regular checks on the calibration were carried out during every mission.

### 4.2 Calibration Overflights

To assure that the LIDAR system is performing within specifications, on a mission-by-mission basis, a snapshot of data is captured over a known surface, most often one of the runways located at the airport of operations. The runway surface is surveyed by collecting hundreds of topographic points using a post-processed kinematic GPS procedure.

At the beginning and end of each mission, two passes are made in opposite directions at right angles to and over the surveyed runway (see Figure 7, below). On average, approximately 50,000 Lidar points are acquired over the runway surface per pass. The surveyed topographic points that define the “known” surface of the runway are used to develop a surface model and the Lidar points from each pass are draped over this model and residuals computed. The residuals from each pass are graphed versus distance along the runway to provide an effective vertical cross-section of the entire Lidar swath at a short moment in time.



**Figure 7. Example of a Calibration Overflight**

The graphs from each pass are used to check that the mirror angle offset and scale, IMU solution roll and pitch bias, and elevation bias are within acceptable tolerances and to finely tune the general parameters on a mission-by-mission basis. This “snapshot” of the Lidar swath also ensures that the system is operating normally and that there are no anomalies contained in the data.

Figure 8 contains a plot of an unbiased runway overflight computed for each mission during the campaign. The following information may be obtained through careful examination of the graphs:

- The 99-percent noise band of the data is consistent at approximately 10 centimeters or less;
- There is no significant mirror scale error (characterized by a smile or frown);

- 
- There is no significant roll error (characterized by a tilt in the noise band);
  - There are no evident data anomalies;

Each plot is accompanied by the average residual and root mean square (RMS) of the residuals for the respective data set. Of the 7 data sets, the largest RMS is 10.3 centimeters, a representation of the combined stochastic noise and bias of the Lidar data prior to any vertical adjustment. The plots and statistics demonstrate that the system is working well within the system specifications.

### **4.3 Additional Calibration**

The runway passes also serve to examine potential pitch and yaw biases that may be present in the data. These parameters are typically more stable over the course of the campaign and are therefore checked only periodically during the acquisition period.

The pitch bias can be analyzed by examining data flown in opposite directions over an area with a sharp change in elevation, such as the edge of a building perpendicular to the direction of flight. A pitch bias will manifest itself as a “ghosting” of the edge of the building by virtue of the opposite direction passes.

In a similar fashion, a bias about the normal axis (yaw) can be detected simply by observing the intensity return images from orthogonal flight lines of obvious features such as the runway center line. A yaw bias will cause a difference in orientation yielding an obvious displacement of visible features.

The results of these various calibration and QC analyses are applied in the mission-by-mission processing of the data to ensure a consistent and accurate overall data set.

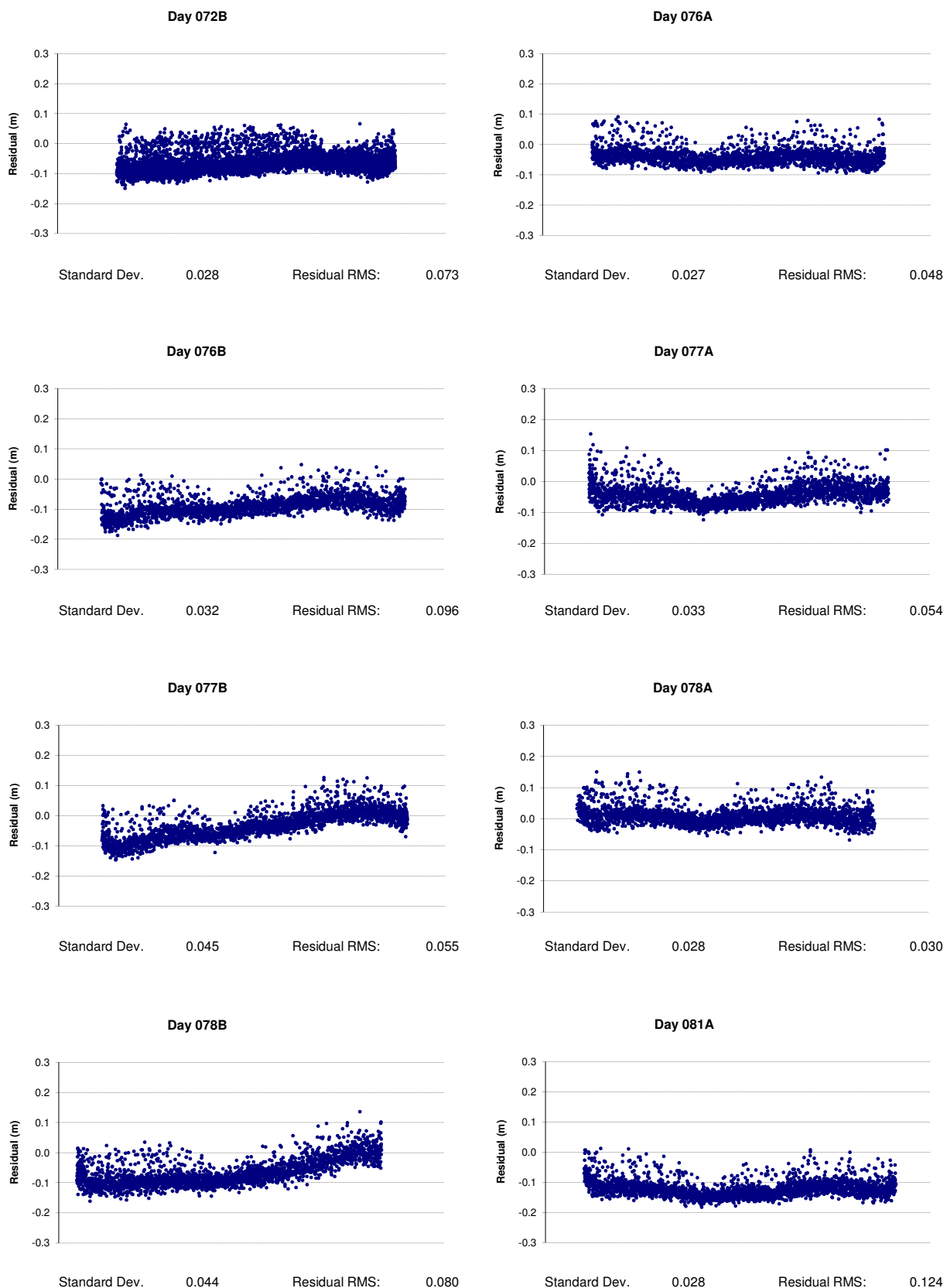
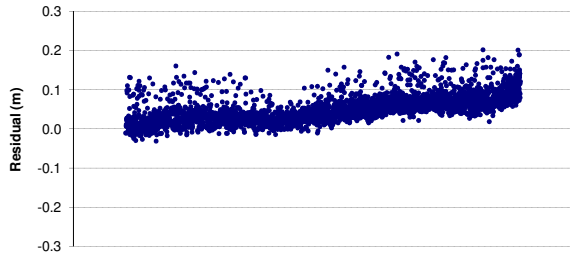


Figure 8. Mission-by-Mission Calibration Pass Analyses

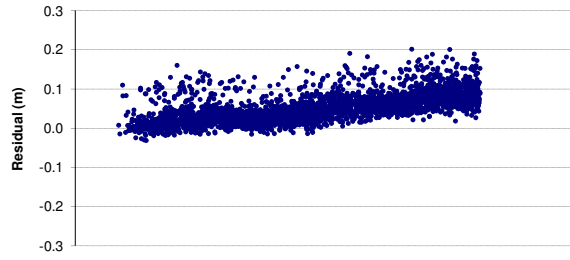


**Day 081B**



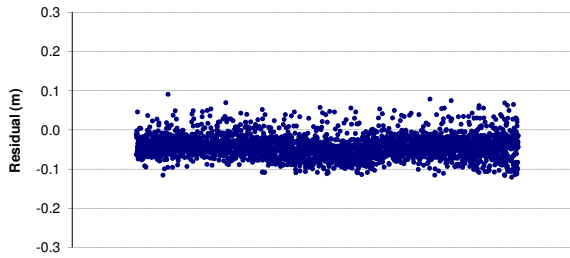
Avg. Residual: 0.035      Residual RMS: 0.062

**Day 082A**



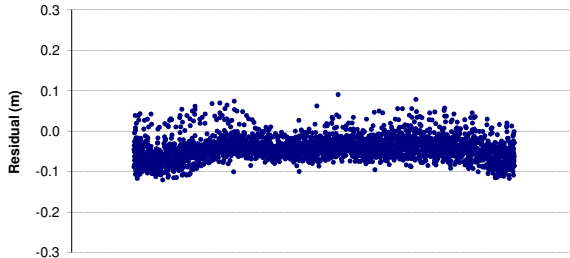
Avg. Residual: 0.028      Residual RMS: 0.059

**Day 083A**



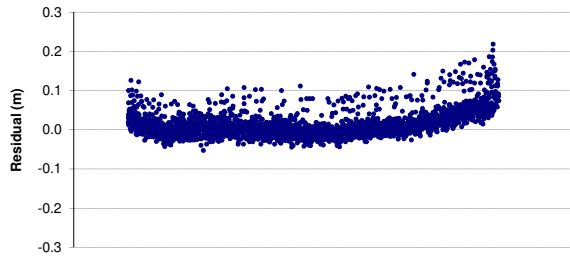
Avg. Residual: 0.027      Residual RMS: 0.040

**Day 083B**



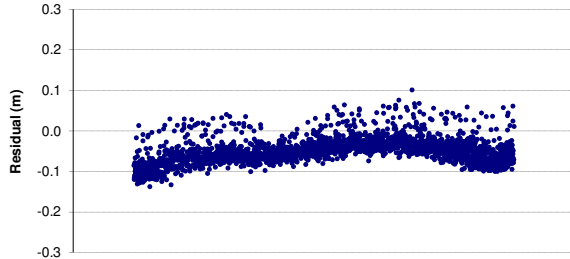
Avg. Residual: 0.028      Residual RMS: 0.051

**Day 084A**



Avg. Residual: 0.033      Residual RMS: 0.037

**Day 084B**



Avg. Residual: 0.031      Residual RMS: 0.058

## 5. LIDAR CHECKPOINT QUALITY ASSURANCE

In a further effort to validate the absolute vertical accuracy of the Lidar-derived elevations, 40 well-distributed check points were established and surveyed with respect to the AGPS base stations and VRS stations that were included in the primary survey network.

In the vicinity of each check point, the post-processed Lidar data is used to generate a surface model upon which the check point is draped. The residual elevation difference is computed at each check point location. Table 3 summarizes the results of the check point analysis in meters. The RMS of all check point differences is 0.064 meters (0.21 foot).

Table 3.

**LIDAR CHECK POINT RESIDUALS**  
Orthometric Elevations (International Feet)

Point	Surveyed Elev.	LIDAR Elev.	Residual
ED1206	797.793	798.060	0.267
ED3342	1214.949	1215.310	0.361
ED3499	883.844	883.530	-0.314
ED3750	701.004	701.570	0.566
KAND	757.195	757.130	-0.065
TC01	957.049	957.110	0.061
TC02	1150.388	1150.510	0.122
TC03	1791.639	1791.730	0.091
TC04	1034.646	1034.900	0.254
TC05	1072.954	1073.520	0.566
TC06	881.871	882.120	0.249
TC07	1083.388	1083.650	0.262
TC08	967.402	967.540	0.138
TC09	1051.346	1051.400	0.054
TC10	2466.743	2466.640	-0.103
TC11	1667.062	1667.400	0.338
TC12	1712.263	1712.340	0.077
TC13	1014.209	1014.400	0.191
TC14	1016.646	1016.730	0.084
TC15	1046.878	1046.940	0.062
TC66	792.433	792.240	-0.193
TC67	777.927	778.030	0.103
TC68	921.852	922.080	0.228
TC69	1034.029	1034.140	0.111
TC70	894.348	894.210	-0.138
TC71	963.029	962.950	-0.079
TC72	821.169	821.040	-0.129
TC73	670.217	670.350	0.133
TC74	914.373	914.250	-0.123
TC75	878.250	878.420	0.170
TC76	671.518	671.160	-0.358
TC77	884.098	884.280	0.182
TC78	910.563	910.700	0.137
TC79	821.033	821.150	0.117
TC80	810.631	810.740	0.109
TC81	849.808	849.830	0.022
TC82	728.208	728.380	0.172
TC83	722.740	722.650	-0.090
TC84	669.682	669.880	0.198
TC85	818.144	817.950	-0.194
TC86	798.170	798.180	0.010
TC87	673.595	673.420	-0.175
TC88	853.486	853.600	0.114
TC89	849.906	849.730	-0.176
TC90	791.532	791.590	0.058
<b>Average</b>			<b>0.077</b>
<b>Standard Deviation</b>			<b>0.197</b>
<b>RMS</b>			<b>0.210</b>



# Appendix I

## Final Coordinates and Elevations

**Dewberry and Davis**  
 SCDNR - Pickens, Oconee, Anderson Counties Campaign Control Survey

**FINAL ADJUSTED COORDINATES**

Horizontal Datum: NAD83(NSRS2007)  
 Epoch: 2007.0  
 Linear Unit: International Meter

Point	Latitude				Longitude				Ellipsoid Height	Standard Deviation		
	g	'	"	'''	g	'	"	'''		$\rho$	$\lambda$	h
<b>Base Stations</b>												
KAND	N	34	29	42.34781	W	82	42	45.15605	200.345	0.005	0.005	0.009
<b>Lidar Check Points</b>												
TC01	N	35	00	10.52261	W	82	39	10.63250	260.672	0.003	0.003	0.007
TC02	N	34	59	17.58088	W	82	45	12.57626	319.776	0.003	0.003	0.007
TC03	N	35	02	51.09587	W	82	48	12.52591	515.586	0.004	0.003	0.008
TC04	N	34	57	42.08142	W	82	49	46.76785	284.504	0.003	0.002	0.007
TC05	N	34	55	51.56148	W	82	53	11.65211	296.170	0.003	0.002	0.007
TC06	N	34	51	29.28686	W	82	51	35.82988	237.814	0.007	0.004	0.013
TC07	N	34	51	33.34039	W	82	46	50.12846	299.196	0.003	0.003	0.007
TC08	N	34	48	09.69043	W	82	58	10.58699	263.957	0.003	0.002	0.006
TC09	N	34	53	17.28557	W	82	58	22.46411	289.665	0.005	0.003	0.009
TC10	N	35	00	51.19849	W	83	01	42.64894	721.922	0.004	0.004	0.011
TC11	N	34	50	33.92833	W	83	07	56.04691	477.748	0.004	0.003	0.010
TC12	N	34	48	00.53035	W	83	16	25.14188	491.788	0.004	0.003	0.007
TC13	N	34	43	15.09403	W	83	09	36.90270	278.530	0.003	0.003	0.007
TC14	N	34	45	45.51225	W	83	03	22.96950	279.097	0.003	0.002	0.006
TC15	N	34	48	56.65335	W	82	36	04.66277	288.029	0.003	0.003	0.009
TC66	N	34	28	12.93263	W	82	25	37.59781	211.224	0.003	0.002	0.007
TC67	N	34	20	30.76928	W	82	41	18.94656	207.168	0.003	0.002	0.005
TC68	N	34	45	46.44216	W	82	45	40.52586	249.990	0.005	0.004	0.008
TC69	N	34	48	40.95960	W	82	48	54.56339	284.168	0.002	0.002	0.006
TC70	N	34	46	35.84301	W	82	40	29.42496	241.591	0.003	0.002	0.006
TC71	N	34	44	46.99847	W	82	35	16.78645	262.550	0.003	0.003	0.006
TC72	N	34	38	32.91566	W	82	46	23.93621	219.453	0.003	0.003	0.008
TC73	N	34	31	27.82752	W	82	48	02.12049	173.722	0.004	0.004	0.009
TC74	N	34	35	45.56956	W	82	58	33.21260	247.981	0.003	0.003	0.007
TC75	N	34	37	38.13963	W	82	54	16.27854	236.870	0.004	0.003	0.007
TC76	N	34	40	54.77249	W	82	51	58.20709	173.754	0.003	0.003	0.008
TC77	N	34	41	33.87358	W	82	58	34.65674	238.608	0.002	0.002	0.006
TC78	N	34	39	18.40943	W	83	05	12.14712	246.796	0.003	0.002	0.006
TC79	N	34	30	39.64098	W	82	59	23.48096	219.701	0.003	0.003	0.006
TC80	N	34	37	11.23501	W	82	28	38.32634	216.340	0.005	0.004	0.009
TC81	N	34	32	47.28095	W	82	34	14.10416	228.458	0.005	0.004	0.011

Point	Latitude				Longitude				Ellipsoid Height	Standard Deviation		
	°	'	"	'''	°	'	"	'''		ρ	λ	h
TC82	N	34	27	17.09452	W	82	36	00.98697	191.670	0.004	0.004	0.012
TC83	N	34	20	49.30431	W	82	36	07.51283	190.353	0.004	0.003	0.009
TC84	N	34	22	34.25252	W	82	48	55.33526	174.013	0.005	0.004	0.009
TC85	N	34	30	40.77149	W	82	45	29.99990	218.861	0.005	0.003	0.009
TC86	N	34	35	26.06462	W	82	45	34.11187	212.555	0.003	0.003	0.008
TC87	N	34	37	18.67416	W	82	51	11.55760	174.486	0.003	0.003	0.006
TC88	N	34	38	22.04457	W	82	41	22.81162	229.331	0.003	0.003	0.008
TC89	N	34	38	15.15589	W	82	35	05.73097	228.265	0.003	0.002	0.008
TC90	N	34	32	19.70924	W	82	39	26.33537	210.693	0.004	0.003	0.006
<b>VRS / CORS Stations</b>												
GVLТ	N	34	49	46.94102	W	82	22	15.37249	250.260	-	-	-
NСHE	N	35	21	21.89186	W	82	30	03.99744	654.460	-	-	-
SCAN	N	34	35	48.45792	W	82	41	00.47566	229.892	-	-	-
SCOE	N	34	40	19.81527	W	83	02	15.01590	232.457	-	-	-
SCPS	N	34	50	13.06816	W	82	40	15.72001	300.783	-	-	-

**NOTES:**

- Ellipsoid heights of the VRS and CORS are to the antenna reference point (ARP)
- Shaded records indicate constrained values



**Dewberry and Davis**  
SCDNR - Pickens, Oconee, Anderson Counties Campaign Control Survey

**FINAL ADJUSTED COORDINATES**

Horizontal Datum: NAD83(NSRS2007)  
Epoch: 2007.0  
Projection: U.S. State Plane South Carolina Zone  
Vertical Datum: NAVD88  
Geoid Model: GEOID09  
Linear Unit: International Foot

Point	Northing (N)	Easting (E)	Elevation (H)	Standard Deviation		
				N	E	H
<i>Base Stations</i>						
KAND	972733.59	1484036.04	757.20	0.01	0.01	0.02
<i>Lidar Check Points</i>						
TC01	1157243.00	1504944.03	957.05	0.01	0.01	0.02
TC02	1152387.02	1474744.52	1150.39	0.01	0.01	0.02
TC03	1174230.45	1460149.53	1791.64	0.01	0.01	0.03
TC04	1143128.08	1451761.04	1034.65	0.01	0.01	0.02
TC05	1132262.05	1434505.72	1072.96	0.01	0.01	0.02
TC06	1105604.30	1442006.10	881.87	0.02	0.01	0.04
TC07	1105594.72	1465819.55	1083.39	0.01	0.01	0.02
TC08	1086037.84	1408728.51	967.40	0.01	0.01	0.02
TC09	1117149.84	1408331.86	1051.35	0.02	0.01	0.03
TC10	1163358.39	1392558.41	2466.75	0.01	0.01	0.04
TC11	1101586.27	1360215.55	1667.07	0.01	0.01	0.03
TC12	1086985.70	1317448.89	1712.27	0.01	0.01	0.02
TC13	1057404.54	1350883.32	1014.21	0.01	0.01	0.02
TC14	1071971.74	1382392.84	1016.65	0.01	0.01	0.02
TC15	1088875.08	1519358.68	1046.88	0.01	0.01	0.03
TC66	962389.57	1569901.91	792.43	0.01	0.01	0.02
TC67	916866.99	1490339.35	777.93	0.01	0.01	0.02
TC68	1070428.61	1471021.57	921.85	0.02	0.01	0.03
TC69	1088349.69	1455144.89	1034.03	0.01	0.01	0.02
TC70	1074990.86	1497054.75	894.35	0.01	0.01	0.02
TC71	1063576.46	1522962.35	963.03	0.01	0.01	0.02



Point	Northing (N)	Easting (E)	Elevation (H)	Standard Deviation		
				N	E	H
TC72	1026669.33	1466648.29	821.17	0.01	0.01	0.03
TC73	983845.46	1457697.56	670.22	0.01	0.01	0.03
TC74	1010860.26	1405403.85	914.37	0.01	0.01	0.02
TC75	1021834.99	1427088.29	878.25	0.01	0.01	0.02
TC76	1041499.56	1438983.23	671.52	0.01	0.01	0.03
TC77	1046067.21	1405956.45	884.10	0.01	0.01	0.02
TC78	1033027.73	1372495.59	910.56	0.01	0.01	0.02
TC79	980020.66	1400607.01	821.03	0.01	0.01	0.02
TC80	1017012.77	1555550.92	810.63	0.02	0.01	0.03
TC81	990746.48	1527086.79	849.81	0.02	0.01	0.04
TC82	957510.22	1517630.94	728.21	0.01	0.01	0.04
TC83	918324.59	1516476.94	722.74	0.01	0.01	0.03
TC84	929995.51	1452298.63	669.68	0.02	0.01	0.03
TC85	978870.25	1470342.00	818.15	0.02	0.01	0.03
TC86	1007711.84	1470488.93	798.17	0.01	0.01	0.03
TC87	1019586.89	1442486.64	673.60	0.01	0.01	0.02
TC88	1025148.99	1491785.75	853.49	0.01	0.01	0.03
TC89	1023953.56	1523277.32	849.91	0.01	0.01	0.03
TC90	988367.49	1500929.04	791.53	0.01	0.01	0.02
<b>VRS / CORS Stations</b>						
GVLТ	1092964.52	1588565.61	923.22	-	-	0.00
NCHE	1285099.58	1552284.65	2246.89	-	-	0.00
SCAN	1009594.26	1493398.83	855.02	-	-	0.00
SCOE	1038939.05	1387412.15	863.70	-	-	0.00
SCPS	1096931.15	1498552.94	1088.75	-	-	0.00

**NOTES:**

- Ellipsoid heights of the VRS and CORS are to the antenna reference point (ARP)
- Shaded records indicate constrained values

# Appendix II

## Fully Constrained Network Adjustment

Horizontal Datum: NAD83(NSRS2007), 2007.0

Vertical Datum: NAVD88





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=====
                SC Base Station Survey - Tricounty; NAD83 VRS Coordinates
Microsearch GeoLab, V2001.9.20.0          GRS 80          UNITS: m,DMS  Page 0001
=====
Fri May 6 10:00:02 2011

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```

Input file:  \\tsclient\J\13639-101 SCDNR\Geolab\Tricounty_Bases_c.iob
Output file: \\tsclient\J\13639-101 SCDNR\Geolab\Tricounty_Bases_c.lst
Options file: C:\Program Files\Microsearch\GeoLab\default.gpj

```

Geoid File: c:\ngs\geoid09\g2009u07pc.gsp

PARAMETERS		OBSERVATIONS	
Description	Number	Description	Number
No. of Stations	6	Directions	0
Coord Parameters	18	Distances	0
Free Latitudes	6	Azimuths	0
Free Longitudes	6	Vertical Angles	0
Free Heights	6	Zenithal Angles	0
Fixed Coordinates	0	Angles	0
Astro. Latitudes	0	Heights	5
Astro. Longitudes	0	Height Differences	0
Geoid Records	0	Auxiliary Params.	0
All Aux. Pars.	0	2-D Coords.	10
Direction Pars.	0	2-D Coord. Diffs.	0
Scale Parameters	0	3-D Coords.	0
Constant Pars.	0	3-D Coord. Diffs.	33
Rotation Pars.	0		
Translation Pars.	0		
-----		-----	
Total Parameters	18	Total Observations	48
Degrees of Freedom =		30	

SUMMARY OF SELECTED OPTIONS

OPTION	SELECTION
Computation Mode	Adjustment
Maximum Iterations	10
Convergence Criterion	0.00100
Residual Rejection Criterion	Tau Max
Confidence Region Types	1D 2D Station Relative
Relative Confidence Regions	Connected Only
Variance Factor (VF) Known	Yes
Scale Covariance Matrix With VF	No
Scale Residual Variances With VF	No
Force Convergence in Max Iters	No
Distances Contribute To Heights	No
Compute Full Inverse	Yes



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=====
                SC Base Station Survey - Tricounty; NAD83 VRS Coordinates
Microsearch GeoLab, V2001.9.20.0          GRS 80          UNITS: m,DMS  Page 0002
=====
Optimize Band Width          | Yes
Generate Initial Coordinates | Yes
Re-Transform Obs After 1st Pass | Yes
Geoid Interpolation Method   | Bi-Quadratic
=====
```



=====  
SC Base Station Survey - Tricounty; NAD83 VRS Coordinates  
Microsearch GeoLab, V2001.9.20.0                      GRS 80                      UNITS: m,DMS                      Page 0003  
=====

Adjusted PLH Coordinates:

CODE	FFF	STATION	LATITUDE		LONGITUDE		ELIP-HEIGHT	
			STD DEV		STD DEV		STD DEV	
PLH	000	GVLN	N 34 49 46.941129	W 82 22 15.372748	250.251	m	0	
			0.005	0.005	0.007			
PLH	000	KAND	N 34 29 42.347812	W 82 42 45.156054	200.345	m	0	
			0.005	0.005	0.009			
PLH	000	NCHE	N 35 21 21.892055	W 82 30 3.997599	654.472	m	0	
			0.005	0.005	0.007			
PLH	000	SCAN	N 34 35 48.457535	W 82 41 0.475655	229.888	m	0	
			0.005	0.005	0.007			
PLH	000	SCOE	N 34 40 19.815403	W 83 2 15.015643	232.452	m	0	
			0.005	0.005	0.006			
PLH	000	SCPS	N 34 50 13.068104	W 82 40 15.719857	300.789	m	0	
			0.005	0.005	0.006			

```

=====
                SC Base Station Survey - Tricounty; NAD83 VRS Coordinates
Microsearch GeoLab, V2001.9.20.0          GRS 80          UNITS: m,DMS  Page 0004
=====

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Adjusted PLO Coordinates:

CODE	FFF	STATION	LATITUDE		LONGITUDE		O-HEIGHT	
			STD DEV		STD DEV		STD DEV	
PLO	000	GVLN	N 34 49	46.941129	W 82 22	15.372748	281.398 m	0
				0.005		0.005	0.007	
PLO	000	KAND	N 34 29	42.347812	W 82 42	45.156054	230.793 m	0
				0.005		0.005	0.009	
PLO	000	NCHE	N 35 21	21.892055	W 82 30	3.997599	684.851 m	0
				0.005		0.005	0.007	
PLO	000	SCAN	N 34 35	48.457535	W 82 41	0.475655	260.611 m	0
				0.005		0.005	0.007	
PLO	000	SCOE	N 34 40	19.815403	W 83 2	15.015643	263.257 m	0
				0.005		0.005	0.006	
PLO	000	SCPS	N 34 50	13.068104	W 82 40	15.719857	331.850 m	0
				0.005		0.005	0.006	

```

=====
                SC Base Station Survey - Tricounty; NAD83 VRS Coordinates
Microsearch GeoLab, V2001.9.20.0                GRS 80                UNITS: m,DMS  Page 0005
=====
Geoid Values:
CODE      STATION      N/S DEFLECTION      E/W DEFLECTION      UNDULATION
-----
GEOI     GVLT             0 0      2.39  0 0      0.81      -31.147 m
GEOI     KAND             0 0      5.61 - 0 0      0.65      -30.449 m
GEOI     NCHE             - 0 0      4.39  0 0      6.77      -30.379 m
GEOI     SCAN             0 0      3.99 - 0 0      0.45      -30.723 m
GEOI     SCOE             0 0      1.64  0 0      1.86      -30.805 m
GEOI     SCPS             0 0      1.37  0 0      0.62      -31.061 m

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=====
                SC Base Station Survey - Tricounty; NAD83 VRS Coordinates
Microsearch GeoLab, V2001.9.20.0                GRS 80                UNITS: m,DMS    Page 0006
=====
Residuals (critical value = 3.214):
NOTE: Observation values shown are reduced to mark-to-mark.

```

TYPE	AT	FROM	TO	OBSERVATION STD DEV	RESIDUAL STD DEV	STD RES PPM
ELAT	SCPS			N 34 50 13.068160 0.010	-0.002 0.005	-0.330
ELON	SCPS			W 82 40 15.720010 0.010	0.004 0.009	0.443
ELAT	SCAN			N 34 35 48.457920 0.010	-0.012 0.005	-2.384
ELON	SCAN			W 82 41 0.475660 0.010	0.000 0.009	0.016
ELAT	SCOE			N 34 40 19.815270 0.010	0.004 0.005	0.816
ELON	SCOE			W 83 02 15.015900 0.010	0.007 0.009	0.748
ELAT	GVLTL			N 34 49 46.941020 0.010	0.003 0.005	0.682
ELON	GVLTL			W 82 22 15.372490 0.010	-0.007 0.009	-0.750
ELAT	NCHE			N 35 21 21.891860 0.010	0.006 0.005	1.299
ELON	NCHE			W 82 30 3.997440 0.010	-0.004 0.009	-0.462
EHGT	SCPS			300.78300 0.010	0.006 0.008	0.776
EHGT	SCAN			229.89200 0.010	-0.004 0.008	-0.534
EHGT	SCOE			232.45700 0.010	-0.005 0.008	-0.651
EHGT	GVLTL			250.26000 0.010	-0.009 0.008	-1.203
EHGT	NCHE			654.46000 0.010	0.012 0.007	1.624
GROUP: 00000, Tri_County_Bases.asc						
DXCT		SCAN	KAND	-1837.45330 0.004	-0.002 0.003	-0.616 0.14
DYCT		SCAN	KAND	-6660.81010 0.010	0.007 0.007	0.991 0.60
DZCT		SCAN	KAND	-9309.31210 0.007	-0.004 0.005	-0.852 0.34
GROUP: 00001, Tri_County_Bases.asc						
DXCT		SCOE	KAND	30964.98360 0.003	0.002 0.002	1.185 0.06
DYCT		SCOE	KAND	-7339.60700 0.008	-0.007 0.005	-1.344 0.19
DZCT		SCOE	KAND	-16191.13990 0.006	0.004 0.003	1.235 0.11
GROUP: 00002, Tri_County_Bases.asc						
DXCT		NCHE	GVLTL	16143.33570	0.001	0.367

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                SC Base Station Survey - Tricounty; NAD83 VRS Coordinates
Microsearch GeoLab, V2001.9.20.0                GRS 80                UNITS: m,DMS                Page 0007
=====
Residuals (critical value = 3.214):
NOTE: Observation values shown are reduced to mark-to-mark.

```

TYPE	AT	FROM	TO	OBSERVATION STD DEV	RESIDUAL STD DEV	STD RES PPM
				0.004	0.003	0.02
DYCT		NCHE	GVLT	-31392.49050	0.002	0.237
DZCT		NCHE	GVLT	-48017.48900	-0.002	-0.258
				0.009	0.008	0.03
GROUP: 00003, Tri_County_Bases.asc						
DXCT		SCPS	SCAN	797.13790	-0.000	-0.116
				0.005	0.004	0.02
DYCT		SCPS	SCAN	-15138.15890	0.006	0.614
				0.011	0.009	0.21
DZCT		SCPS	SCAN	-21941.44480	-0.003	-0.357
				0.010	0.009	0.12
GROUP: 00004, Tri_County_Bases.asc						
DXCT		NCHE	SCPS	-11129.41980	0.001	0.150
				0.005	0.003	0.01
DYCT		NCHE	SCPS	-34550.84560	0.000	0.033
				0.009	0.007	0.00
DZCT		NCHE	SCPS	-47327.73670	0.004	0.727
				0.008	0.006	0.07
GROUP: 00005, Tri_County_Bases.asc						
DXCT		SCPS	SCOE	-32005.30390	0.001	0.229
				0.005	0.004	0.02
DYCT		SCPS	SCOE	-14459.35220	0.009	1.274
				0.009	0.007	0.24
DZCT		SCPS	SCOE	-15059.62210	-0.006	-0.952
				0.008	0.006	0.16
GROUP: 00006, Tri_County_Bases.asc						
DXCT		GVLT	SCPS	-27272.75000	-0.006	-1.036
				0.007	0.006	0.22
DYCT		GVLT	SCPS	-3158.38460	0.028	2.529
				0.012	0.011	1.02
DZCT		GVLT	SCPS	689.77660	-0.018	-2.980
				0.008	0.006	0.65
GROUP: 00007, Tri_County_Bases.asc						
DXCT		NCHE	SCOE	-43134.72390	0.002	0.323
				0.006	0.005	0.02
DYCT		NCHE	SCOE	-49010.16570	-0.023	-2.172
				0.012	0.010	0.25
DZCT		NCHE	SCOE	-62387.38150	0.021	1.800
				0.013	0.012	0.23
GROUP: 00008, Tri_County_Bases.asc						
DXCT		SCOE	SCAN	32802.44400	-0.003	-0.939
				0.005	0.004	0.10
DYCT		SCOE	SCAN	-678.82300	0.013	1.427
				0.010	0.009	0.37
DZCT		SCOE	SCAN	-6881.81100	-0.009	-0.922

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                SC Base Station Survey - Tricounty; NAD83 VRS Coordinates
Microsearch GeoLab, V2001.9.20.0          GRS 80          UNITS: m,DMS  Page 0008
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Residuals (critical value = 3.214):

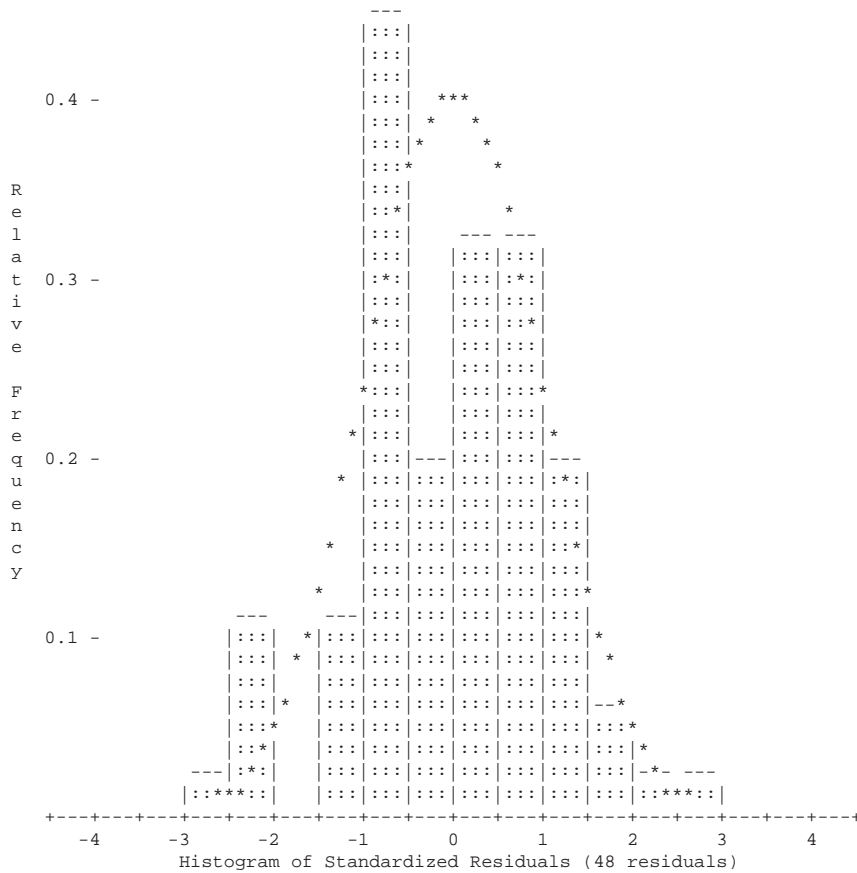
NOTE: Observation values shown are reduced to mark-to-mark.

TYPE AT	FROM	TO	OBSERVATION STD DEV	RESIDUAL STD DEV	STD RES PPM
			0.011	0.009	0.26
GROUP: 00009, Tri_County_Bases.asc					
DXCT	SCAN	GVLN	26475.62130	-0.003	-0.787
			0.005	0.003	0.07
DYCT	SCAN	GVLN	18296.49280	0.017	2.087
			0.010	0.008	0.45
DZCT	SCAN	GVLN	21251.70790	-0.019	-2.109
			0.010	0.009	0.48
GROUP: 00010, Tri_County_Bases.asc					
DXCT	GVLN	KAND	-28313.06760	-0.006	-0.869
			0.008	0.007	0.12
DYCT	GVLN	KAND	-24957.32650	0.013	0.646
			0.022	0.021	0.27
DZCT	GVLN	KAND	-30560.99840	-0.007	-0.583
			0.013	0.012	0.14

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                SC Base Station Survey - Tricounty; NAD83 VRS Coordinates
Microsearch GeoLab, V2001.9.20.0          GRS 80          UNITS: m,DMS  Page 0009
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SC Base Station Survey - Tricounty; NAD83 VRS Coordinates  
Microsearch GeoLab, V2001.9.20.0                      GRS 80                      UNITS: m,DMS                      Page 0010

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S T A T I S T I C S                      S U M M A R Y

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Residual Critical Value Type	Tau Max
Residual Critical Value	3.2141
Number of Flagged Residuals	0
Convergence Criterion	0.0010
Final Iteration Counter Value	2
Confidence Level Used	95.0000
Estimated Variance Factor	0.8691
Number of Degrees of Freedom	30

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Chi-Square Test on the Variance Factor:

5.5501e-01 < 1.0000 < 1.5529e+00 ?

THE TEST PASSES

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NOTE: All confidence regions were computed using the following factors:

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Variance factor used	=	1.0000
1-D expansion factor	=	1.9600
2-D expansion factor	=	2.4477

Note that, for relative confidence regions, precisions are computed from the ratio of the major semi-axis and the spatial distance between the two stations.

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                SC Base Station Survey - Tricounty; NAD83 VRS Coordinates
Microsearch GeoLab, V2001.9.20.0                GRS 80                UNITS: m,DMS  Page 0011
=====
2-D and 1-D Station Confidence Regions (95.000 and 95.000 percent):
STATION          MAJOR SEMI-AXIS  AZ      MINOR SEMI-AXIS          VERTICAL
-----
GVLT              0.012 163          0.012          0.013
KAND              0.013 74           0.012          0.017
NCHE              0.012 25           0.012          0.013
SCAN              0.012 39           0.012          0.013
SCOE              0.012 44           0.012          0.013
SCPS              0.012 28           0.012          0.012

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                SC Base Station Survey - Tricounty; NAD83 VRS Coordinates
Microsearch GeoLab, V2001.9.20.0                GRS 80                UNITS: m,DMS  Page 0012
=====
2-D and 1-D Relative Station Confidence Regions (95.000 and 95.000 percent):
FROM      TO      MAJ-SEMI  AZ  MIN-SEMI  VERTICAL  DISTANCE  PPM
-----
GVLТ      KAND      0.008    55    0.008    0.018    48564.109  0.17
GVLТ      NCHE      0.008    16    0.007    0.015    59596.771  0.14
GVLТ      SCAN     0.008   178    0.007    0.015    38566.241  0.20
GVLТ      SCPS     0.008   169    0.007    0.014    27463.689  0.28
KAND      SCAN     0.007    81    0.005    0.016    11593.356  0.57
KAND      SCOE     0.006    82    0.004    0.015    35705.086  0.17
NCHE      SCOE     0.009    24    0.008    0.015    90303.851  0.10
NCHE      SCPS     0.009    29    0.007    0.014    59645.110  0.14
SCAN     SCOE     0.006    76    0.005    0.013    33523.430  0.19
SCAN     SCPS     0.008    27    0.007    0.014    26668.826  0.28
SCOE     SCPS     0.008    34    0.006    0.013    38212.621  0.21

```